Dover Pedestrian Plan – Pedestrian Signals Feasibility Study

Prepared for:

Dover/Kent County MPO

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INTRODUCTION

The Dover Pedestrian Signals Feasibility Study consists of evaluating five (5) existing signalized intersections in the City of Dover to determine if it is feasible and sensible to install pedestrian signals at these locations. This Study is the result of a recommendation from the Dover Pedestrian Plan Update. The Study does not include any design work, but rather focuses on existing intersection geometry, field conditions, vehicular and pedestrian volumes, and nearby land use to determine where pedestrian signals would be logical and feasible, and to analyze their impact on vehicular traffic.

The five study intersections included in this study are as follows:

- 1. US Route 13 & Roosevelt Avenue
- 2. US Route 13 & South State Street
- 3. US Route 13 & Webbs Lane
- 4. South State Street & Webbs Lane
- 5. Governors Avenue/State Street & Walker Road

A field view of each study intersection was conducted and the following conditions were evaluated:

- Intersection geometry, lane configurations, speed limits, signing and striping
- Existing pedestrian and bicycle facilities (if any), including worn paths, sidewalks, curb ramps, crosswalks and bike lanes
- Signal phasing and timings / traffic operations
- Vehicle and pedestrian volumes for weekday AM and PM peak hours
- Nearby land use
- Possible crosswalk and pedestrian signal layout options

Existing traffic counts for the study intersections are compiled in *Appendix A* while existing signal timings provided by DelDOT can be found in *Appendix B*.

Upon gathering the necessary information and conducting field views, the traffic impacts of the proposed pedestrian signals were analyzed using Critical Movement Summation (CMS) analysis. This basic analysis tool provides the critical lane volume, level of service for the overall intersection, and required green time for different scenarios based on traffic volumes and signal phasing. To account for pedestrians in the CMS analysis, an equivalent number of vehicles are essentially added to the phase during which pedestrians would cross in order to provide the appropriate amount of green time for that phase to safely cross pedestrians. Artificially inflating the vehicular volumes to account for the required pedestrian crossing time thereby degrades the level of service and allows the analyst to quantitatively assess how the addition of pedestrian signals would affect traffic operations. Using this methodology, the CMS analysis presents a true worst-case scenario in which a pedestrian call is made during each cycle. In reality, based on the traffic count data and nearby land use, the study intersections typically would not receive much more than 4 or 5 pedestrian calls per hour. The CMS analysis worksheets can be found in *Appendix C*.

Based on field observations, evaluation of nearby land use and traffic impact analysis, it is recommended that pedestrian signals be added to each of the study intersections with the exception of US Route 13 and South State Street. The evaluation and analysis for each intersection is outlined in the following study.

US ROUTE 13 & ROOSEVELT AVENUE

Existing Intersection Geometry & Traffic Operations

The skewed intersection of US 13 & Roosevelt Ave has two through lanes and exclusive left and right-turn lanes for both directions of US 13. The eastbound and westbound Roosevelt Avenue approaches each contain one left-turn lane and one shared through/right turn lane. The right turns are channelized by a concrete median on the eastbound approach and by a painted median on the westbound approach. Acceleration lanes do not exist for these movements. Northbound and southbound lefts from US 13 operate concurrently and are protected-permitted with flashing red arrows. All of the minor street movements operate concurrently. The posted speed limit on both approaches of US 13 is 50 MPH and the posted speed limit on both approaches of Roosevelt Avenue is 25 MPH.

Existing Pedestrian Facilities

There are currently no pedestrian crossing accommodations at this intersection. Sidewalk exists at the southwest corner along the site frontage of an office/medical building, but the sidewalk along southbound US 13 ends at the entrance to the building roughly 300' from the intersection with Roosevelt Ave. The sidewalk along Roosevelt Avenue continues for about 750' west, but ends at a driveway before reaching the intersection with State Street. Sidewalk also exists along the site frontage of a car dealership on the southeast corner of the intersection. This sidewalk is limited to the dealership site and does not tie in with any existing pedestrian facilities along US 13 or Roosevelt Ave. Similarly, there is sidewalk along the northeast corner at the car wash site frontage, extending north along northbound US 13 and an adjacent strip mall. The sidewalk ends at the intersection with Evergreen Drive and picks back up past the used auto dealer along the McDonald's site frontage. The sidewalk ends at the end of the McDonald's property. No sidewalk or existing pedestrian facilities exist at the northwest corner of the intersection of US 13 & Roosevelt Avenue.

Nearby Land Use

The general land use along the US 13 corridor in this area is mainly commercial and retail, although none of the businesses are big generators of pedestrian activity. The area to the west of US 13 is mainly residential behind the strip malls, and there is a small pocket of housing to the east of US 13, bound by the St. Jones River and Puncheon Run Connector. Although there is a McDonald's, a diner, and a liquor store close to the intersection of US 13 and Roosevelt Avenue, the overall feel is more commercial with the adjacent auto dealers and office buildings, and not much pedestrian activity is present as evidenced by the traffic count data and field observations.

Potential Pedestrian Facilities

Since there are already existing pedestrian facilities along the northbound side of US 13, it would be feasible to provide a pedestrian crossing with pedestrian signals and upgraded curb ramps across the east leg of Roosevelt Avenue, although there are virtually no pedestrian destinations south of the car dealership. The other logical pedestrian crossing location is across the south leg of US 13, which would provide a link between the various businesses and residences on each side of US 13. There are also existing sidewalks on both sides of the south leg to tie in to. Three potential scenarios were analyzed for the south leg crossing: a single-stage crossing with the existing concurrent east-west phasing where westbound left turns must yield to pedestrians; a two-stage crossing with an exclusive pedestrian phase for crossing of the southbound US 13 lanes in order to eliminate the conflict with pedestrians and westbound left turns; and a two-stage crossing with split phasing for the east and west legs

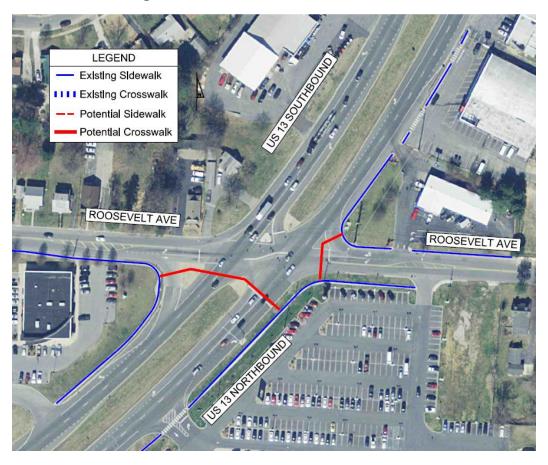


Figure 1 – US Route 13 and Roosevelt Avenue

Critical Movement Summation (CMS) Analysis

Table 1: US Route 13 & Roosevelt Avenue CMS Analysis Results

Scenario	Level of Service (Critica	al Movement Volume)
Scenario	AM Peak Hour	PM Peak Hour
Existing Conditions (No Peds)	A (682)	A (771)
With Pedestrians (1 stage crossing)	F (2,170)	F (2,081)
With Pedestrians (2 stage crossing w/ exclusive ped phase)	F (1,813)	F (1,779)
With Pedestrians (2 stage crossing w/ split phase)	D (1,411)	D (1,449)

Recommendations

A crosswalk with pedestrian signals can be added to the eastern leg of the intersection, linking the existing sidewalk that runs along US 13 northbound on both sides of Roosevelt Boulevard. Adding a pedestrian crossing to this leg has no effect on traffic operations as the green time for US 13 is more than enough to serve the minimum pedestrian crossing time.

Although the count data shows little pedestrian activity in this area, a pedestrian crossing of US 13 is recommended at this location based on the adjacent land use and the lack of nearby crossing locations. There are residential neighborhoods and various businesses on both sides of US 13 near the study intersection, however, the closest intersection along US 13 to the south is with State Street, where the addition of pedestrian facilities is not recommended due to the size, layout and lack of pedestrian infrastructure in the vicinity. In the northbound direction, pedestrians would have to walk approximately two-thirds of a mile to the intersection with River Road/Public Safety Boulevard in order to cross US 13.

Analysis of the different crossing scenarios resulted in severe service level degradation for the single-stage crossing option and the two-stage crossing with an exclusive pedestrian phase option. The green time required to cross pedestrians across US 13 severely impacts the heavy through movements on US 13 and drops the overall intersection level of service from an A to an F for both scenarios. On the other hand, converting the signal to split phase operation with a two-stage pedestrian crossing only degraded the level of service from an A to a D. In reality, the number of pedestrian calls would be very low and the worst-case results that the CMS analysis produces are not a good representation of how the signal would typically operate. Permanently changing the minor street phasing from concurrent to split-phase would take green time away from US 13 traffic during each cycle, and would have an overall worse effect on the intersection than one long pedestrian crossing once or twice an hour. Further coordination is required to determine the best course of action for this particular crossing.

US ROUTE 13 & SOUTH STATE STREET

Existing Intersection Geometry & Traffic Operations

The massive, skewed intersection of US 13 & South State Street is oriented in such a way that South State Street runs due north/south, and thus, US 13 is considered an east/west roadway for the discussion of this intersection. The eastbound and westbound approaches of US 13 each contain two through lanes, two left-turn lanes and a channelized right-turn lane. The left turn lanes and right turn lanes are channelized from the through lanes by concrete islands at both approaches. The northbound and southbound South State Street approaches each contain one left-turn lane, two-through lanes and one channelized right turn lane separated by a concrete island. The dual left turns from eastbound and westbound US 13 are protected-only and run concurrently, and the left turns from northbound and southbound South State Street are protected-only and run concurrently, as well. The posted speed limit on both approaches of US 13 is 50 MPH. The posted speed limit on the northbound approach of South State Street is 40 MPH, and the posted speed limit on the southbound approach of South State Street is 35 MPH.

Existing Pedestrian Facilities

There are currently no pedestrian crossing accommodations at this intersection. A paved path exists along the southbound direction of South State Street and runs from the intersection of South State Street and Webbs Lane/Wolf Creek Boulevard, north under the Puncheon Run Connector overpass, and terminating at the southwest corner of the intersection of South State Street and US 13. The path dead-ends at the intersection of South State Street and US 13 and does not connect to anything as there are no other pedestrian facilities at this intersection. North/west of US 13, the sidewalk along southbound South State Street ends approximately 350 feet before reaching the intersection with US 13. No worn paths or evidence of pedestrian activity were noticed during field observations.

Nearby Land Use

The general land use at this intersection is commercial and essentially no pedestrian destinations exist on the eastbound side of US 13. The residential area is mainly north and west of this intersection and pedestrians would utilize the Governors Avenue corridor, which has sidewalks on both sides of the street, to reach any of the various businesses and fast food establishments along the southbound direction of US 13 near the Webbs Lane intersection, which are the nearest pedestrian attractions.

Potential Pedestrian Facilities

The installation of pedestrian crossings at the intersection of US 13 and South State Street are not feasible. The sheer size of the intersection and the skewed geometry would require very long pedestrian routes across many lanes of travel, which would negatively affect the level of service at the very busy intersection and promote a safety hazard to pedestrians, even with a legal signalized crossing. In addition, there are no nearby existing pedestrian facilities to tie in to, and no nearby pedestrian generating businesses. Safer, more accessible pedestrian routes on the lower volume local streets can be utilized for the residential area to the north and west of this intersection.



Figure 2 – US Route 13 & South State Street

Critical Movement Summation (CMS) Analysis

CMS analysis was not performed on the intersection of US 13 & South State Street given the infeasibility of adding pedestrian facilities to this intersection.

Recommendations

The installation of pedestrian crossings or pedestrian signals at this intersection is not recommended.

US ROUTE 13 & WEBBS LANE

Existing Intersection Geometry & Traffic Operations

The intersection of US 13 & Webbs Lane contains two through lanes, one left-turn lane and one right-turn lane in both directions of US 13. The left turn lanes and right turn lanes are channelized from the through lanes by concrete islands at both approaches. The eastbound Webbs Lane approach contains one left-turn lane, one shared through/left-turn lane and one right-turn lane with a channelizing concrete island. The westbound Webbs Lane approach contains one left-turn lane and one shared through/right-turn lane with a concrete channelizing island. The left turns from northbound and southbound US 13 operate concurrently and are protected-permitted with flashing red arrows. The eastbound and westbound movements are split-phased. The posted speed limit on both approaches of US 13 is 50 MPH, and the posted speed limit on the westbound approach of Webbs Lane is 40 MPH. On the eastbound Webbs Lane approach, the posted speed limit is 35 MPH west of Governors Ave. There is no posted speed limit east of Governors Avenue, but the westbound Webbs Lane departure from US 13 has a posted speed limit of 25 MPH.

Existing Pedestrian Facilities

There are currently no pedestrian crossing accommodations at this intersection. Sidewalk exists on the northwest corner of the intersection along the Rite Aid site frontage, which ties into the pedestrian facilities at the intersection of Webbs Lane and South Governors Ave, which was recently upgraded to provide pedestrian crossings across all four legs complete with ADA compliant curb ramps and pedestrian signals. Existing sidewalk extends west along both sides of Webbs Lane and south and north along both sides of South Governors Ave in to the residential area. Besides the sidewalk along the Rite Aid frontage, no other pedestrian facilities exist at the intersection of US 13 and Webbs Lane. During a 20 minute field visit, two pedestrians were observed illegally crossing US 13 from the west to east. Two fatal pedestrian crashes have recently occurred near this intersection – one in June 2014 and one in November 2014. In both cases, the pedestrians were illegally crossing US 13 at night when they were fatally struck.

Nearby Land Use

The land use near this intersection is generally retail and residential. Although there is essentially nothing along the northbound side of US 13, the southbound side contains several pedestrian attractions including a drug store, two fast-food restaurants and a gas station/convenience store. Located behind these businesses west of US 13 is the Rodney Village neighborhood, which contains a comprehensive network of sidewalks that connect the residential area to South Governors Avenue, providing walkable access to the various businesses along southbound US 13. Located east of US 13 and south of Webbs Lane is the Kent Acres neighborhood, as well as additional residences along Taylor Drive located behind the various businesses along northbound US 13. Pedestrians who want to cross US 13 from these residential neighborhoods located east of the major highway to access the businesses on the west side of the highway have no way to legally and safely cross US 13.

Potential Pedestrian Facilities

The most sensible location for a pedestrian crossing at the intersection of US 13 and Webbs Lane is along the southern leg of the intersection, which would provide a legal crossing for pedestrians originating in the residential area east of US 13. This would be a long crossing distance for a single pedestrian phase, so a pedestrian refuge island in the US 13 median may be required in conjunction with a two-phase pedestrian crossing phase. Assuming that a pedestrian crossing is installed along the southern leg, it would also be beneficial to extend the existing sidewalk on the south side of Webbs Lane west of US 13 that currently ends at the Wendy's entrance, so that it connects with the proposed curb ramp at the southwest corner of US 13 & Webbs Lane. Similarly, new sidewalk could be installed on the south side

of Webbs Lane, east of US 13 to connect the Kent Acres neighborhood to the proposed curb ramp and crossing at the southeast corner of US 13 & Webbs Lane. With the installation of a pedestrian crossing across the south leg, it would also be feasible to provide a crossing of the west leg of Webbs Lane since sidewalk already exists on the northwest corner.

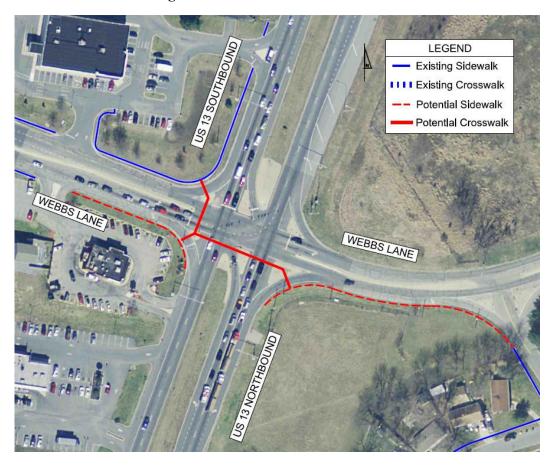


Figure 3 – US Route 13 & Webbs Lane

Critical Movement Summation (CMS) Analysis

Table 2: US Route 13 & Webbs Lane CMS Analysis Results

Caanania	Level of Service (Critica	al Movement Volume)
Scenario	AM Peak Hour	PM Peak Hour
Existing Conditions (No Peds)	A (947)	C (1,229)
With Pedestrians (1 stage crossing)	D (1,391)	E (1,452)
With Pedestrians (2 stage crossing)	C (1,243)	D (1,316)

Recommendations

Signalized pedestrian crossings can be added to the south and west legs of the intersection of US Route 13 and Webbs Lane. Providing a pedestrian crossing of US 13 at Webbs Lane is important given the history of pedestrian crashes at this location, as well as the existing land use which features residential areas on the east side of US 13 and various stores and restaurants on the west side of US 13. South of the

US 13 and Webbs Lane intersection, the nearest pedestrian crossing of US 13 is 2 miles away at the intersection of Voshells Mill Star Hill Road. Meanwhile, there is no way for pedestrians to cross US 13 north of Webbs Lane due to the Puncheon Run Connector ramps and the intersection with South State Street, which do not contain pedestrian facilities. Adding a single-stage pedestrian crossing of US 13 does add significant delay to the intersection, but the CMS service level still remains at E or better for both the AM and PM peak hours. Breaking the crossing into a two-phase crossing helps reduce this delay. The addition of a crossing across the west leg does not impact the traffic operations given the long green time for US 13, and it provides a link to the businesses on either side of Webbs Lane. Implementation of these pedestrian improvements would require extension of the sidewalk along the south side of Webbs Lane on both sides of US 13, in order to tie-in to existing pedestrian facilities. Curb ramps and pedestrian signals would need to be constructed on the northwest, southwest and southeast corners, and modifications to the concrete channelizing islands would be required for ADA compliance. It should be noted that a future US 13 Safety Project will add a third through lane to US 13 in both directions at this intersection, and may include new pedestrian signals among other facilities. The US 13 Safety Project is still in its infancy and design is not scheduled to begin until FY 2017 with construction likely starting a minimum of six years later. It is still recommended that pedestrian signals be installed in the interim given the long duration of the design and construction process, and the uncertainty of funding and scheduling at this point.

SOUTH STATE STREET & WEBBS LANE/WOLF CREEK BOULEVARD

Existing Intersection Geometry & Traffic Operations

The intersection of South State Street & Webbs Lane contains one through lane, one left-turn lane and one right-turn lane on both South State Street approaches, and contains one shared through/left-turn lane and one right-turn lane on the eastbound Webbs Lane approach and the westbound Wolf Creek Boulevard approach. The right turn lanes contain concrete channelization islands, except at the northbound State Street approach. The left turns from northbound and southbound State Street operate concurrently and are protected-permitted. The eastbound and westbound movements are split-phased. The posted speed limit on both approaches on South State Street is 40 MPH. There are no posted speed limit signs on the eastbound Webbs Lane approach or the westbound Wolf Creek Boulevard approach, but the departure lanes have posted speed limits of 40 MPH and 25 MPH, respectively.

Existing Pedestrian Facilities

There is one marked crosswalk with curb ramps crossing the east leg (Wolf Creek Boulevard) of the intersection, but no pedestrian signals exist. Sidewalk exists along the northbound side of South State Street and both sides of Wolf Creek Boulevard which extend in to the office complex. On the southbound side of State Street is a paved path that ends at the northwest corner of the intersection of Webbs Lane and South State Street. There are no pedestrian facilities on the northwest or southwest corners of the intersection or along the eastbound Webbs Lane approach.

Nearby Land Use

The land use near this intersection is generally residential, especially south of Webbs Lane/Wolf Creek Boulevard. There is an office complex on the northeast corner of the intersection and a separate office complex near the northwest corner of the intersection/westbound Webbs Lane. In general, there is not much in the area of the intersection that would attract pedestrians, besides those who may live nearby and walk to work at one of the office complexes.

Potential Pedestrian Facilities

The existing pedestrian crossing across the east leg of the intersection could be upgraded with the installation of pedestrian signals, which would improve safety for pedestrians. A pedestrian crossing of South State Street could also be added. The most sensible location for this crossing is along the northern leg of the intersection, which would connect the sidewalk along Wolf Creek Boulevard and the office complex frontage to the paved path along southbound State Street.



Figure 4 – South State Street & Webbs Lane

Critical Movement Summation (CMS) Analysis

Table 3: South State Street & Webbs Lane CMS Analysis Results

Scenario	Level of Service (Critica	al Movement Volume)
Scenario	AM Peak Hour	PM Peak Hour
Existing Conditions (No Peds)	A (954)	C (1,183)
With Pedestrians (1 stage crossing)	C (1,175)	C (1,285)

Recommendations

A crosswalk with pedestrian signals can be added to the northern leg of the intersection, and pedestrian signals can be installed for the existing east leg crossing. Although there does not appear to be much pedestrian activity in this area, the new north leg crossing would link the pedestrian facilities throughout the office complex and adjacent neighborhood to the east of State Street with the business complex and paved path on the west side of State Street. The addition of two signalized pedestrian crossings in the CMS analysis did not have much of an impact on the level of service. Implementation of these pedestrian improvements would require installation of pedestrian signals and new sidewalk/curb ramps on the northwest corner of the intersection, which would tie in to the existing paved path.

NORTH STATE STREET, NORTH GOVERNORS AVENUE & WALKER ROAD

Existing Intersection Geometry & Traffic Operations

The signalized intersection of North State Street, North Governors Avenue and Walker Road has an odd geometric layout. The northbound State Street approach contains one through lane and one left-turn lane for the left onto Walker Road or the sharp left onto Governors Avenue. The southbound State Street approach contains two lanes: one through lane for State Street and one right-turn lane for the slight right onto Governors Avenue and the standard right to Walker Road. The eastbound Walker Road approach contains one left-turn lane and one right-turn lane that serves the sharp right onto Governors Avenue and the standard right onto North State Street. The northeast approach of North Governors Avenue contains one through lane that serves the movement to northbound State Street. The northbound through movement on North State Street is a free movement, while the northbound State Street left turn movement is protected only. The right lane of southbound State Street has a dedicated signal that serves movements to Governors Avenue and Walker Road, which runs concurrently with the northeast-bound Governors Avenue movement to State Street (the left turn from Governors Avenue to Walker Road is prohibited at all times). The left lane of southbound State Street also has a designated signal and phase that serves the southbound through movement to State Street. The westbound Walker Road approach also has a separate phase that serves all movements. The speed limit on northbound and southbound State Street is 35 MPH, the Walker Road speed limit is 30 MPH and the Governors Avenue speed limit is 25 MPH.

Existing Pedestrian Facilities

Sidewalk exists along the northbound side of North State Street and continues north across the bridge over Silver Lake. Sidewalk also exists along the southbound side of State Street across the bridge and continues along westbound Walker Road. There is also sidewalk along westbound Walker Road which connects with sidewalk running along southbound Governors Avenue. There is sidewalk along northbound Governor's Avenue and southbound State Street, but they connect south of the intersection and do not extend up to the intersection. There are no pedestrian crossings across any of the four legs of the intersection.

Nearby Land Use

The land use near this intersection is residential, especially south west of the intersection. There is a cemetery east of State Street and Silver Lake separates the residential area from the US 13 commercial corridor to the north/east of the intersection area. A park is also located near the northwest corner of the intersection between Walker Road and Silver Lake. Nearby pedestrian attractions include stores and restaurants north of Silver Lake, an office complex a few blocks west down Walker Road, and a dense residential area along with Wesley College and downtown Dover businesses and offices located less than one mile to the south.

Potential Pedestrian Facilities

Given the intersection geometry and nearby land use, the most sensible location for a pedestrian crossing is across the Walker Road leg and the North Governors Avenue leg. The northbound and southbound State Street approaches are not ideal for pedestrian crossings since the northbound through movement is free and the northern leg contains four travel lanes. It is assumed that pedestrians can cross North State Street a block or two south of the intersection in the low speed, residential area that only contains one lane in each direction. The Governors Avenue crosswalk would require extending the sidewalks along southbound State Street and northbound Governors Avenue up to the intersection, along with new curb ramps at the corner of Governors Avenue and State Street and Governors Avenue and Walker Road. The new curb ramp at Governors Avenue and Walker Road would also serve the proposed Walker Road

crossing, which would connect the residential area to the south to the lake and park to the north. The proposed Walker Road crosswalk presents a sight-distance issue where southbound State Street motorists have limited vision due to the trees and fence on the northwest corner of the intersection along the lake. In order to safely cross pedestrians across Walker Road, the southbound right-turn movement would need to be restricted via a separate phase. Another option is to add a separate pedestrian phase for both of the proposed crossings.



Figure 5 – North State Street, North Governors Avenue & Walker Road

Critical Movement Summation (CMS) Analysis

Table 4: North State Street, North Governors Ave, & Walker Road CMS Analysis Results

Scenario	Level of Service (Critica	al Movement Volume)
Scenario	AM Peak Hour	PM Peak Hour
Existing Conditions (No Peds)	B (1,004)	B (1,016)
With Pedestrians (1 stage crossing)	C (1,214)	D (1,359)

Recommendations

Crosswalks can be added to the Governors Avenue leg and Walker Road leg of the intersection in order to provide safe pedestrian access from the heavily residential area to the south and to Richardson Park and the US 13 commercial corridor to the north. To safely cross pedestrians across the Walker Road leg, a separate pedestrian phase would be required. The separate pedestrian phase was taken into account in the CMS analysis, and the level of service was moderately impacted. Implementation of these pedestrian improvements would require the extension of the sidewalks along Governors Avenue and North State Street up to the intersection, and construction of curb ramps and pedestrian signals for the two proposed crossings. Other signal modifications may be necessary to accommodate the proposed phasing.

Dover Pedestrian Signals Feasibility Study

Appendix A

Traffic Count Data

610 466-1469 TSTData.com

Location: Dover, Delaware

Intersection: Rt 13/Roosevelt Ave

Date: Tuesday, May 20, 2014

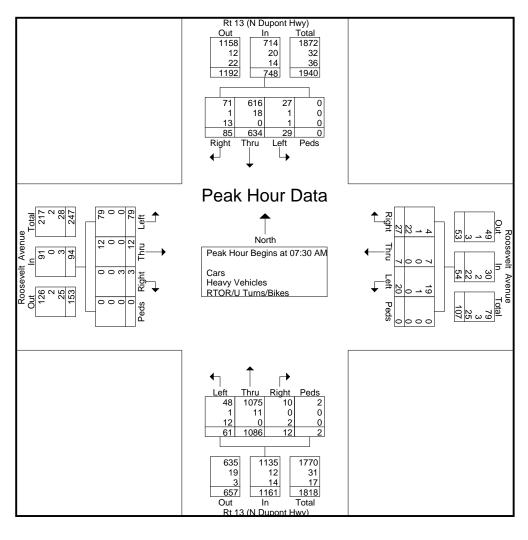
Counter: WC LD

File Name: Dover Weekday 22

Site Code : 22

Start Date : 5/20/2014

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		Sc	outhbou	nd			W	estbou	nd			N	orthbou	ınd			E	astbour	nd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Ana	alysis F	rom 07	:00 AM	to 11:4:	5 AM - I	Peak 1 c	of 1			.,							•			**	
Peak Hour for	Entire I	Intersec	tion Be	gins at 0	7:30 AN	Л															
07:30 AM	24	178	10	0	212	4	1	6	0	11	2	278	13	0	293	2	1	18	0	21	537
07:45 AM	25	155	6	0	186	14	2	5	0	21	1	317	27	0	345	1	3	23	0	27	579
08:00 AM	15	119	6	0	140	6	0	4	0	10	6	256	7	2	271	0	4	19	0	23	444
08:15 AM	21	182	7	0	210	3	4	5	0	12	3	235	14	0	252	0	4	19	0	23	497
Total Volume	85	634	29	0	748	27	7	20	0	54	12	1086	61	2	1161	3	12	79	0	94	2057
% App. Total	11.4	84.8	3.9	0		50	13	37	0		1	93.5	5.3	0.2		3.2	12.8	84	0		
PHF	.850	.871	.725	.000	.882	.482	.438	.833	.000	.643	.500	.856	.565	.250	.841	.375	.750	.859	.000	.870	.888
Cars	71	616	27	0	714	4	7	19	0	30	10	1075									
% Cars	83.5	97.2	93.1	0	95.5	14.8	100	95.0	0	55.6	83.3	99.0	78.7	100	97.8	0	100	100	0	96.8	95.8
Heavy Vehicles	1	18	1	0	20	1	0	1	0	2	0	11	1	0	12	0	0	0	0	0	34
% Heavy Vehicles	1.2	2.8	3.4	0	2.7	3.7	0	5.0	0	3.7	0	1.0	1.6	0	1.0	0	0	0	0	0	1.7
RTOR/U Turns/Bikes	13	0	1	0	14	22	0	0	0	22	2	0	12	0	14	3	0	0	0	3	53
% RTOR/U Turns/Bikes	15.3	0	3.4	0	1.9	81.5	0	0	0	40.7	16.7	0	19.7	0	1.2	100	0	0	0	3.2	2.6



610 466-1469 TSTData.com

Location: Dover, Delaware

Intersection: Rt 13/Roosevelt Ave

Date: Tuesday, May 20, 2014

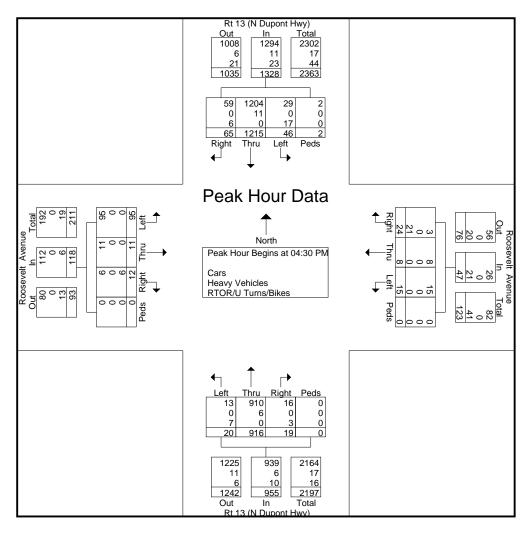
Counter: WC LD

File Name: Dover Weekday 22

Site Code : 22

Start Date : 5/20/2014

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Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Ana	alysis F	rom 12	:00 PM	to 05:43	5 PM - P	eak 1 of	1														
Peak Hour for	Entire 1	Intersec	tion Beg	gins at ()4:30 PM	1															
04:30 PM	16	337	6	0	359	10	0	2	0	12	3	206	4	0	213	4	1	29	0	34	618
04:45 PM	18	308	15	0	341	2	3	3	0	8	4	239	5	0	248	3	3	23	0	29	626
05:00 PM	17	286	11	0	314	7	1	3	0	11	8	263	5	0	276	3	5	22	0	30	631
05:15 PM	14	284	14	2	314	5	4	7	0	16	4	208	6	0	218	2	2	21	0	25	573
Total Volume	65	1215	46	2	1328	24	8	15	0	47	19	916	20	0	955	12	11	95	0	118	2448
% App. Total	4.9	91.5	3.5	0.2		51.1	17	31.9	0		2	95.9	2.1	0		10.2	9.3	80.5	0		
PHF	.903	.901	.767	.250	.925	.600	.500	.536	.000	.734	.594	.871	.833	.000	.865	.750	.550	.819	.000	.868	.970
Cars	59	1204																			
% Cars	90.8	99.1	63.0	100	97.4	12.5	100	100	0	55.3	84.2	99.3	65.0	0	98.3	50.0	100	100	0	94.9	96.9
Heavy Vehicles	0	11	0	0	11	0	0	0	0	0	0	6	0	0	6	0	0	0	0	0	17
% Heavy Vehicles	0	0.9	0	0	0.8	0	0	0	0	0	0	0.7	0	0	0.6	0	0	0	0	0	0.7
RTOR/U Turns/Bikes	6	0	17	0	23	21	0	0	0	21	3	0	7	0	10	6	0	0	0	6	60
% RTOR/U Turns/Bikes	9.2	0	37.0	0	1.7	87.5	0	0	0	44.7	15.8	0	35.0	0	1.0	50.0	0	0	0	5.1	2.5



610 466-1469 TSTData.com

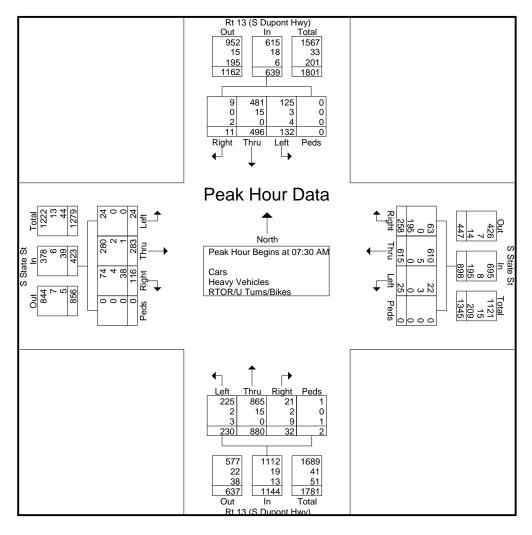
Location: Dover, Delaware File Name: Dover Weekday 23

Intersection: Rte 13/S State St Site Code : 23

Date: Wednesday, May 21, 2014 Start Date: 5/21/2014

Counter: WC LD Page No : 4

]	Rt 13 (S	Dupor	nt Hwy)		S	State S	St]	Rt 13 (S	S Dupo	nt Hwy)		S	State S	St		
		So	uthbou	nd			W	estbou	nd			N	orthbou	ınd			E	astbour	nd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Ana	alysis F	rom 07:	00 AM	to 11:4	5 AM - I	Peak 1 c	of 1														
Peak Hour for	Entire 1	ntersect	tion Beg	gins at (07:30 AN	1															
07:30 AM	1	140	27	0	168	73	161	6	0	240	7	226	52	1	286	16	50	5	0	71	765
07:45 AM	3	110	40	0	153	76	184	5	0	265	12	257	84	0	353	42	81	6	0	129	900
08:00 AM	4	114	28	0	146	43	133	4	0	180	10	206	50	1	267	30	94	9	0	133	726
08:15 AM	3	132	37	0	172	66	137	10	0	213	3	191	44	0	238	28	58	4	0	90	713
Total Volume	11	496	132	0	639	258	615	25	0	898	32	880	230	2	1144	116	283	24	0	423	3104
% App. Total	1.7	77.6	20.7	0		28.7	68.5	2.8	0		2.8	76.9	20.1	0.2		27.4	66.9	5.7	0		
PHF	.688	.886	.825	.000	.929	.849	.836	.625	.000	.847	.667	.856	.685	.500	.810	.690	.753	.667	.000	.795	.862
Cars	9	481	125	0	615	63	610	22	0	695	21	865	225	1	1112	74	280	24	0	378	2800
% Cars	81.8	97.0	94.7	0	96.2	24.4	99.2	88.0	0	77.4	65.6	98.3	97.8	50.0	97.2	63.8	98.9	100	0	89.4	90.2
Heavy Vehicles																					
% Heavy Vehicles	0	3.0	2.3	0	2.8	0	0.8	12.0	0	0.9	6.3	1.7	0.9	0	1.7	3.4	0.7	0	0	1.4	1.6
RTOR/U Turns/Bikes	2	0	4	0	6	195	0	0	0	195	9	0	3	1	13	38	1	0	0	39	253
% RTOR/U Turns/Bikes	18.2	0	3.0	0	0.9	75.6	0	0	0	21.7	28.1	0	1.3	50.0	1.1	32.8	0.4	0	0	9.2	8.2



610 466-1469 TSTData.com

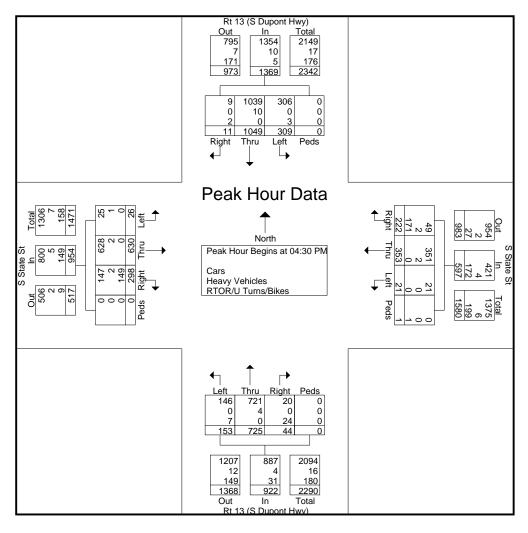
Location: Dover, Delaware File Name: Dover Weekday 23

Intersection: Rte 13/S State St Site Code : 23

Date: Wednesday, May 21, 2014 Start Date: 5/21/2014

Counter: WC LD Page No : 5

	I	Rt 13 (S So	S Dupor outhbou)			State S]	,	S Dupo	nt Hwy ind)			State S astbour			
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Ana	alysis F	rom 12:	00 PM	to 05:4:	5 PM - P	eak 1 o	f 1														
Peak Hour for	Entire I	ntersec	tion Beg	gins at ()4:30 PM	1															
04:30 PM	2	314	82	0	398	43	89	3	0	135	6	200	30	0	236	77	151	6	0	234	1003
04:45 PM	0	231	80	0	311	68	94	6	1	169	13	170	46	0	229	75	184	6	0	265	974
05:00 PM	4	255	70	0	329	62	90	6	0	158	20	186	33	0	239	85	149	8	0	242	968
05:15 PM	5	249	77	0	331	49	80	6	0	135	5	169	44	0	218	61	146	6	0	213	897
Total Volume	11	1049	309	0	1369	222	353	21	1	597	44	725	153	0	922	298	630	26	0	954	3842
% App. Total	0.8	76.6	22.6	0		37.2	59.1	3.5	0.2		4.8	78.6	16.6	0		31.2	66	2.7	0		
PHF	.550	.835	.942	.000	.860	.816	.939	.875	.250	.883	.550	.906	.832	.000	.964	.876	.856	.813	.000	.900	.958
Cars	9	1039																			
% Cars	81.8	99.0	99.0	0	98.9	22.1	99.4	100	0	70.5	45.5	99.4	95.4	0	96.2	49.3	99.7	96.2	0	83.9	90.1
Heavy Vehicles	0	10	0	0	10	2	2	0	0	4	0	4	0	0	4	2	2	1	0	5	23
% Heavy Vehicles	0	1.0	0	0	0.7	0.9	0.6	0	0	0.7	0	0.6	0	0	0.4	0.7	0.3	3.8	0	0.5	0.6
RTOR/U Turns/Bikes	2	0	3	0	5	171	0	0	1	172	24	0	7	0	31	149	0	0	0	149	357
% RTOR/U Turns/Bikes	18.2	0	1.0	0	0.4	77.0	0	0	100	28.8	54.5	0	4.6	0	3.4	50.0	0	0	0	15.6	9.3



610 466-1469 TSTData.com

Location: Dover, Delaware

Intersection: US 13/Webb's Lane Date: Wednesday, June 4th, 2014

Date. Wednesday, Julie 4th, 2014

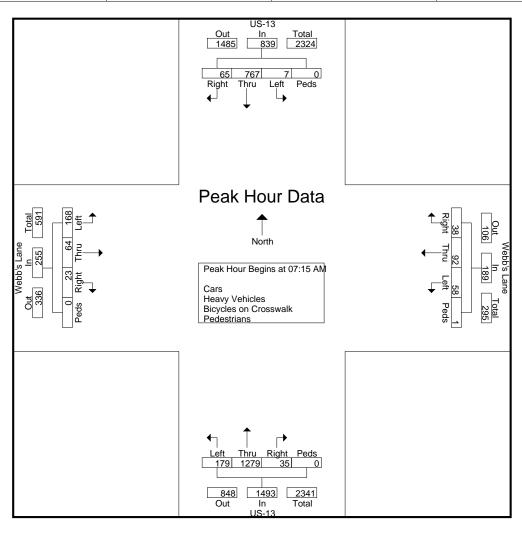
Counter: MIO CT

File Name: Dover Weekday 26

Site Code : 26

Start Date : 6/4/2014

			US-13	3			We	ebb's L	ane				US-13	3			We	ebb's L	ane]
		So	uthbou	und			W	estbou	und			N	orthbo	und			E	<u>astbou</u>	ınd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Ar	nalysis	From 0	7:00 A	M to 1	1:45 AM	1 - Peal	k 1 of 1														
Peak Hour fo	r Entire	Inters	ection I	Begins	at 07:1	5 AM															
07:15 AM	11	180	1	0	192	13	17	13	0	43	7	285	38	0	330	4	15	47	0	66	631
07:30 AM	16	210	1	0	227	9	15	12	0	36	13	325	54	0	392	9	11	40	0	60	715
07:45 AM	25	200	1	0	226	12	29	22	0	63	13	353	52	0	418	5	20	46	0	71	778
MA 00:80	13	177	4	0	194	4	31	11	1	47	2	316	35	0	353	5	18	35	0	58	652
Total Volume	65	767	7	0	839	38	92	58	1	189	35	1279	179	0	1493	23	64	168	0	255	2776
% App. Total	7.7	91.4	0.8	0		20.1	48.7	30.7	0.5		2.3	85.7	12	0		9	25.1	65.9	0		
PHF	.650	.913	.438	.000	.924	.731	.742	.659	.250	.750	.673	.906	.829	.000	.893	.639	.800	.894	.000	.898	.892



610 466-1469 TSTData.com

Location: Dover, Delaware

Intersection: US 13/Webb's Lane

Date: Wednesday, June 4th, 2014

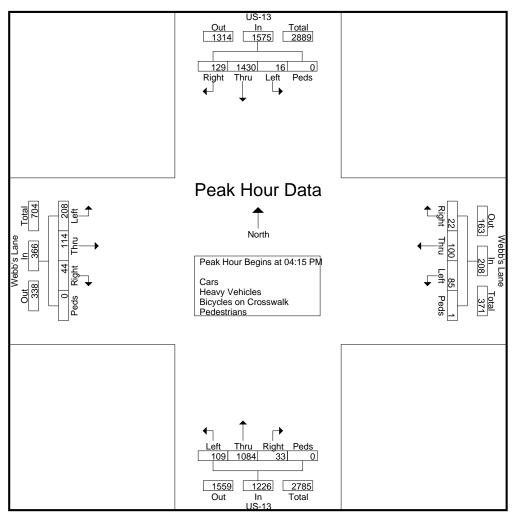
Counter: MIO CT

File Name: Dover Weekday 26

Site Code : 26

Start Date : 6/4/2014

		Sc	US-13					bb's L				N	US-13					ebb's L astbou			
Start Time	Rig ht	Thr	Left	Ped	App. Total	Rig ht	Thr	Left	Ped	App. Total	Right	Thr u	Left	Peds	App. Total	Right	Thr u	Left	Peds	App. Total	Int. Total
Peak Hour Ar	nalysis	From ²	12:00 F	M to 0	5:45 PN	l - Peak	1 of 1														
Peak Hour fo	r Entire	Inters	ection	Begins	at 04:15	5 PM															
04:15 PM	27	348	3	0	378	2	22	21	0	45	5	249	33	0	287	11	22	31	0	64	774
04:30 PM	39	372	2	0	413	0	27	20	0	47	6	223	21	0	250	10	35	53	0	98	808
04:45 PM	28	371	6	0	405	0	27	21	0	48	1	325	34	0	360	13	32	62	0	107	920
05:00 PM	35	339	5	0	379	20	24	23	1	68	21	287	21	0	329	10	25	62	0	97	873
Total Volume	129	1430	16	0	1575	22	100	85	1	208	33	1084	109	0	1226	44	114	208	0	366	3375
% App. Total	8.2	90.8	1	0		10.6	48.1	40.9	0.5		2.7	88.4	8.9	0		12	31.1	56.8	0		
PHF	.827	.961	.667	.000	.953	.275	.926	.924	.250	.765	.393	.834	.801	.000	.851	.846	.814	.839	.000	.855	.917



DelDOT- Traffic Management Center

169 Brick Store Landing Road Smyrna, DE 19977 Telephone: 302-659-4066

Counter No.: T-2923

Counted By: Heather Mantz Weather: Daylight/Clear/Dry Day of Week: Wednesday File Name: S.StateSt-WebbsLane

Site Code : 00000000 Start Date : 10/5/2011

			te Stree				0.70	/olf Cre	77.07				te Stree					s Lane astbou			
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Tota
Peak Hour Analys	is From 0	6:00 AM	to 08:45	AM - P	eak 1 of 1																
Peak Hour for Ent	tire Interse	ection Be	gins at 0	7:15 AN	Λ																
07:15 AM	4	63	2	0	69	4	1	1	0	6	0	149	35	1	185	15	2	5	0	22	282
07:30 AM	3	70	12	0	85	6	6	5	0	17	1	202	42	2	247	29	1	5	0	35	384
07:45 AM	6	76	7	0	89	10	11	7	1	29	6	261	30	0	297	71	3	6	0	80	498
08:00 AM	5	65	15	0	85	20	9	4	0	33	7	187	26	1	221	64	15	11	0	90	429
Total Volume	18	274	36	0	328	40	27	17	1	85	14	799	133	4	950	179	21	27	0	227	1590
% App. Total	5.5	83.5	11	0		47.1	31.8	20	1.2		1.5	84.1	14	0.4		78.9	9.3	11.9	0		
PHF	.750	.901	.600	.000	.921	.500	.614	.607	.250	.644	.500	.765	.792	.500	.800	.630	.350	.614	.000	.631	.80

DelDOT- Traffic Management Center

169 Brick Store Landing Road Smyrna, DE 19977 Telephone: 302-659-4066

Counter No.: T-2923

Counted By: Heather Mantz Weather: Daylight/Clear/Dry Day of Week: Wednesday File Name: S.StateSt-WebbsLane

Site Code : 00000000 Start Date : 10/5/2011

	S. State Street (K25) Southbound						olf Cre			S. State Street (K25) Webbs Lane (K19: Northbound Eastbound)2)					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	is From 0	3:00 PM	to 05:45	PM - P	eak 1 of 1																
Peak Hour for Ent																					
04:45 PM	21	258	10	0	289	12	11	7	0	30	2	154	32	0	188	36	3	6	0	45	552
05:00 PM	10	234	4	0	248	18	12	8	0	38	1	98	30	0	129	46	5	7	0	58	473
05:15 PM	8	232	9	0	249	13	16	6	0	35	2	126	29	0	157	43	6	5	0	54	495
05:30 PM	6	187	7	0	200	7	8	5	1	21	1	112	44	0	157	46	2	5	0	53	431
Total Volume	45	911	30	0	986	50	47	26	1	124	6	490	135	0	631	171	16	23	0	210	1951
% App. Total	4.6	92.4	3	0		40.3	37.9	21	0.8	(0.00)	1	77.7	21.4	0	######################################	81.4	7.6	11	0		15.50
PHF	.536	.883	.750	.000	.853	.694	.734	.813	.250	.816	.750	.795	.767	.000	.839	.929	.667	.821	.000	.905	.884

610 466-1469 TSTData.com

Location: Dover, Delaware

Intersection: State St/Walker Rd

Date: Tuesday, May 20, 2014

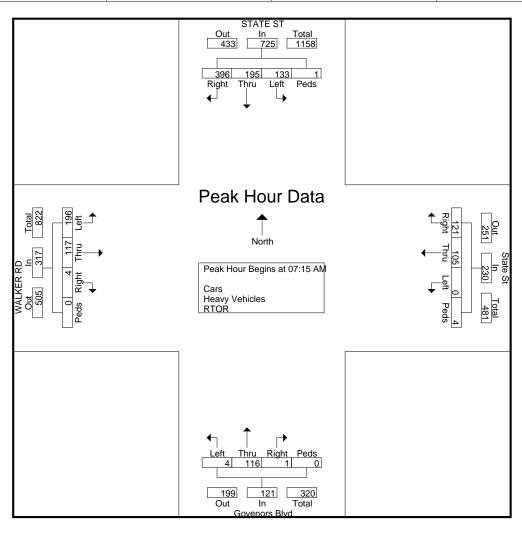
Counter: RZ

File Name: Dover Weekday 43

Site Code: 43

Start Date : 5/20/2014

		S	TATE	ST			,	State S	St			Gov	/enors	Blvd		٧	VALKE	R RD]
		Sc	outhbo	und			W	<u>estbou</u>	und			N	orthbo	und			E	astbou	und		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Ar	nalysis	From (07:00 A	AM to 1	1:45 AN	1 - Peal	k 1 of 1														
Peak Hour fo	r Entire	Inters	ection	Begins	at 07:1	5 AM															
07:15 AM	84	39	32	0	155	25	26	0	3	54	0	22	0	0	22	0	50	57	0	107	338
07:30 AM	86	58	41	1	186	31	30	0	1	62	0	27	4	0	31	2	28	47	0	77	356
07:45 AM	120	52	30	0	202	37	30	0	0	67	0	38	0	0	38	2	23	54	0	79	386
08:00 AM	106	46	30	0	182	28	19	0	0	47	1	29	0	0	30	0	16	38	0	54	313
Total Volume	396	195	133	1	725	121	105	0	4	230	1	116	4	0	121	4	117	196	0	317	1393
% App. Total	54.6	26.9	18.3	0.1		52.6	45.7	0	1.7		0.8	95.9	3.3	0		1.3	36.9	61.8	0		
PHF	.825	.841	.811	.250	.897	.818	.875	.000	.333	.858	.250	.763	.250	.000	.796	.500	.585	.860	.000	.741	.902



610 466-1469 TSTData.com

Location: Dover, Delaware

Intersection: State St/Walker Rd

Date: Tuesday, May 20, 2014

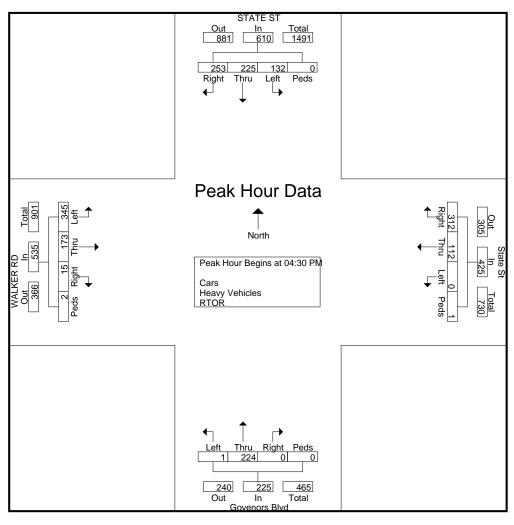
Counter: RZ

File Name: Dover Weekday 43

Site Code: 43

Start Date : 5/20/2014

		_	TATE :	_				State S					enors			V	VALKE E	R RD			
Start Time	Rig ht	Thr	Left	Ped s	App. Total	Rig ht	Thr	Left	Ped s	App. Total	Right	Thr u	Left	Peds	App. Total	Right	Thr u	Left	Peds	App. Total	Int. Total
Peak Hour Ar	nalysis	From	12:00 F	M to 0	5:45 PN	l - Peak	1 of 1														
Peak Hour fo	r Entire	Inters	ection	Begins	at 04:30) PM															
04:30 PM	62	60	30	0	152	77	24	0	0	101	0	60	0	0	60	1	65	93	0	159	472
04:45 PM	62	53	34	0	149	92	38	0	1	131	0	65	1	0	66	5	28	81	2	116	462
05:00 PM	63	56	27	0	146	63	28	0	0	91	0	52	0	0	52	1	40	104	0	145	434
05:15 PM	66	56	41	0	163	80	22	0	0	102	0	47	0	0	47	8	40	67	0	115	427
Total Volume	253	225	132	0	610	312	112	0	1	425	0	224	1	0	225	15	173	345	2	535	1795
% App. Total	41.5	36.9	21.6	0		73.4	26.4	0	0.2		0	99.6	0.4	0		2.8	32.3	64.5	0.4		
PHF	.958	.938	.805	.000	.936	.848	.737	.000	.250	.811	.000	.862	.250	.000	.852	.469	.665	.829	.250	.841	.951



Dover Pedestrian Signals Feasibility Study

Appendix B

Signal Timings

PERMIT # K148

LOCATION OF SIGNAL: US 13 & ROOSEVELT AVE

DATE OF CHART: 4/19/2005
DATE INSTALLED: 9/12/1985
CONTROLLER TYPE: EPAC
COORDINATION TYPE: ACTRA
COORDINATION ADDRESS: 2

CHART REVISION: A.1
CABINET TYPE: PCOM
MONITOR TYPE: NEMA+

COORDINATION ADDRESS: 2						į.		
PHASE NUMBER	1	2	3	4	5	6	7	8
PHASE LOCATION	SBLT	NB RT 13		EB.	NB LT	SB RT 13		WB
MIN GRN	5	10		5	5	10		5
PASS/10	30	50	1000	40	30	50		40
MAX #1	30	60		30	30	60		30
MAX #2	30	60		30	30	60	that the best	× 30
YEL/10	50	50		40	50	50		40
RED/10	20	20		20	-20	20	95 (16.98)	20
145 IV								
AINI/10	ora Great	0		0	-0	0		as 0
MAX INI	0	0		0	0	0		0
TIMBEF = 100	0	0		0	0	0		0
CAR BEF	0	0		0	0	0		0
TIME TO A Page 148, as 145, as a common of the same and t	0	0.44	2.0	0	0	4 = ₹0		0
MGAP/10	0	0		0	0	0		0
WALK	0.4	-15		0.4	0.55	15	0.06	0
PED CLR	0	15		0	0	15		0
EXT PCL	a (10)	0	4.5 (1.5)	0,	. 0	4 0 Z	344	, U
• • •								
INITIAL	1	4	0	1. 1.	1 1	4	0	0
NA RESP	0	0		0	0	0	<u> </u>	0
								0 -
V. RECALL AND THE SECOND	0	2		0	0	2		0
P. RECALL	0	2		0	0	2	L	
				December 1997		T 0	1	10
NL MEM.	1.1	0		0				1
2 ENTRY	0	11		11	0	<u> </u>	1	
					C=SBFL	r	D= NB FL	
OVERLAP/LOCATION	Δ=		B=		C=SDFL		בן וישואו דעו	
PHASES			<u> </u>		1		<u> </u>	

** INTERSECTION NOTES **

RED LIGHT RUNNING CAMERA LOCATION	
T-HEAD [FLASHING LEFT TURN ARROWS] INSTALLED ON SBLT & NBLTS	
PREEMPT, TRANS.= NB & SB, OR EB & WB	
	_
THIS TIMESHEET REFLECTS A CHANGE TO ACTRA SIGNAL SYSTEM NO TIMING ADJUSTMENTS	S
TO YELLOW OR RED TIMES	

TOD by Zone

ZONE/GROUP: K035 US13 LOCHMEATH WAY > ROOSI

Everyda	Time of Day Changes for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
00:01	Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
Weekday	Time of Day Changes for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
06:30	Pattern Change to 331 - 180 NB for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
09:00	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
15:30	Pattern Change to 311 - 180 SB for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
18:30	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
21:00	Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
Weekend	Time of Day Changes for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

09:00	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
18:30	Pattern Change to 141 - 120 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
21:00	Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

Dial 1/Split 1

Phase	1	2	3	4	5	6	7	8
Time	14	58	0	18	14	58	0	18
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18		12	13	18		12
Min Ped Time	0	30		0	0	30		0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Dial 1/Split 4

Phase	1	2	3	4	5	6	7	8
Time	18	78	0	24	18	78	0	24
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18		12	13	18		12
Min Ped Time	0	30		0	0	30		0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	61	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 2/Split 2

Phase	1	2	3	4	5	6	7	8
Time	21	103	0	26	21	103	0	26
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18		12	13	18		12
Min Ped Time	0	30		0	0	30		0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	143	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 3/Split 1

Phase	1	2	3	4	5	6	7	8
Time	15	140	0	25	18	137	0	25
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18		12	13	18		12
Min Ped Time	0	30		0	0	30		0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	52	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 3/Split 3

Phase	1	2	3	4	5	6	7	8
Time	15	140	0	25	18	137	0	25
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18		12	13	18		12
Min Ped Time	0	30		0	0	30		0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	173	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

INTERSECTION TIMESHETT PACKET NOTICE TO PROCEED

Signal Pern Date of Tin Controller Coordination	nesheet: <u> </u>	PAC		R M C B	evision Numb	Der: B, Z NEMA+ Address: 3 1200	OTH <u>3.78</u>	<u>12 31</u>
			Pł	nase Data				
PHASE # PHASE LOCATION	SB LT	2 NB US13	EB LT	4 WB	NB LT	6 5B US13	7 WB LT	8 <i>E</i> B
MIN GRN	5	10		_5_	_5_	10	<u> </u>	_5_
PASS/10	40	50	<u>30</u>	40	40	50	<u>30</u>	40
MAXI	20	45	20	25	20	45	20	<u>25</u>
MAX II	20	45	20	25	20	45	20	25
YEL/10	40	50	40	50	40	50	40	50
RED/10	20	20	20	20	20	20	20	20
AINI/10		****					B100	
MAX IN		*****				· · · · · · · · · · · · · · · · · · ·		
CAR BEF					-			
REDUCE					····		*****	
MGAP/10				***				
WALK								
PED CLR			**************************************		<u></u>			#-11-1-1-14-1B
EXT PCL								
INITIAL		4						
NA RESP								
V. RECALL		2				_2	- Alla	
P. RECALL				# =========				***************************************
NL MEM								
2 ENTRY SPCL SEQ OMIT Ø					State of the state			1
OCAL Ø								
OCAL		wallen was a service of the service						
	•		OVER	LAP DATA				· · · · · ·
OVERLAP LO	CATION	Α		В		<u>C</u>		
OVERLAP PI **DO NOT US **UNDER CO	SE TIMES	SHEET**						
\sqrt{OKTOU}	Z <i>lo 13</i> ISE TI <u>M</u> E		l	1	Signal	Permit Number	er_ <i>K113</i>	

TOD by Zone

ZONE/GROUP: K035 US13 LOCHMEATH WAY > ROOSI

Everyday	Time of Day	Changes for	K035 US13	LOCHMEATH	WAY > ROOSEVELT AVE
TO A CT A CUST A	I IIII OI Day	Changes for	Trody Cold	LOCITIVILATII	WAI - ROODL VELL AVE

00:01 Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

Weekdays Time of Day Changes for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

06:30	Pattern Change to 331 - 180 NB for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
09:00	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
15:30	Pattern Change to 311 - 180 SB for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
18:30	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
21:00	Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

Weekends Time of Day Changes for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

09:00	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
18:30	Pattern Change to 141 - 120 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
21:00	Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

Dial 1/Split 1

Phase	1	2	3	4	5	6	7	8
Time	12	42	12	24	12	42	12	24
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	12	18	12	13	12	18	12	13
Min Ped Time	0	0	0	0	0	0	0	0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time		·		, i				

Dial 1/Split 4

Phase	1	2	3	4	5	6	7	8
Time	20	58	12	30	18	60	12	25
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	12	18	12	13	12	18	12	13
Min Ped Time	0	0	0	0	0	0	0	0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	66	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 2/Split 2

Phase	1	2	3	4	5	6	7	8
Time	24	79	13	34	30	72	14	34
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	12	18	12	13	12	18	12	13
Min Ped Time	0	0	0	0	0	0	0	0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time						·		

Offset	1	2	3
Time	149	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 3/Split 1

Phase	1	2	3	4	5	6	7	8
Time	25	94	13	48	22	95	15	48
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	12	18	12	13	12	18	12	13
Min Ped Time	0	0	0	0	0	0	0	0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	83	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 3/Split 3

Phase	1	2	3	4	5	6	7	8
Time	25	81	15	59	35	71	15	59
Mode	0 - AP	1 - CP	0 - AP	0 - AP	3 - XR	1 - CP	0 - AP	0 - AP
Min Veh Time	12	18	12	13	12	18	12	13
Min Ped Time	0	0	0	0	0	0	0	0

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	156	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

INTERSECTION TIMESHEET PACKET NOTICE TO PROCEED

Date of Tim Controller T	nit Number:	-02-04 AC		Ro M	ocation: <u>US</u> evision Numb onitor Type: _ oordination A	er: <u> </u>	WeBBS LI	1 . ;
			Pha	ise Data			1	
PHASE # PHASE		2	3	4	5	6	7	8
LOCATION	SBLT	<u>nB</u>	EB	WB	nbur	SB		
MIN GRN	_5_	10	5	5	_5_	10		<u> </u>
PASS/10	30	50	40	40	30	50		
MAXI	30	40	<u>30</u>	30	<u>30</u>	60		
MAX II	<u>30</u>	60	30	<u> 30</u>	30	60		
YEL/10	<u>_5</u>	50	40	40	50	50		·
RED/10	20	20	<u>aD</u>	20	20	20		
AINI/10				, mare a comment			•	
MAX INI		**************************************				******************************		
CAR BEF							-	
REDUCE					*		***************************************	
MGAP/10		**************************************	***************************************	400000000000000000000000000000000000000				
WALK		15				<u> 15</u>	· .	
PED CLR		15				15		
EXT PCL		0				_ 0_		
INITIAL	<u>l</u>	<u> </u>				4		
NA RESP		0				D		
V. RECALL		2				2	Water the second	
P. RECALL		2				_2_		
NL MEM					_1_			
2 ENTRY								
			OVERI	AP DATA				
		A		В		C	D)
OVERLAP LO					50	FLT	na)	<u> </u>

TOD by Zone

ZONE/GROUP: K035 US13 LOCHMEATH WAY > ROOSI

Everyday	Time of Day Changes for	· K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
Everyuay	Time of Day Changes for	ROSS USIS LUCIIVILATII WAT > ROOSE VELI AVE

00:01 Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

Weekdays Time of Day Changes for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

06:30	Pattern Change to 331 - 180 NB for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
09:00	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
15:30	Pattern Change to 311 - 180 SB for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
18:30	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
21:00	Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

Weekends Time of Day Changes for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

09:00	Pattern Change to 221 - 150 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
18:30	Pattern Change to 141 - 120 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE
21:00	Pattern Change to 111 - 90 BAL for K035 US13 LOCHMEATH WAY > ROOSEVELT AVE

Dial 1/Split 1

Phase	1	2	3	4	5	6	7	8
Time	14	46	15	15	14	46	0	0
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18	12	12	13	18		
Min Ped Time	0	30	0	0	0	30		

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Dial 1/Split 4

Phase	1	2	3	4	5	6	7	8
Time	15	67	20	18	18	64	0	0
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18	12	12	13	18		
Min Ped Time	0	30	0	0	0	30		

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	9	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 2/Split 2

Phase	1	2	3	4	5	6	7	8
Time	23	80	27	20	24	79	0	0
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18	12	12	13	18		
Min Ped Time	0	30	0	0	0	30		

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time								
Min Ped Time								

Offset	1	2	3
Time	67	0	0
Mode	0 - Normal	0 - Normai	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 3/Split 1

Phase	1	2	3	4	5	6	7	8
Time	18	112	29	21	24	106	0	0
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18	12	12	13	18		
Min Ped Time	0	30	0	0	0	30		

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time			-					
Min Ped Time		,						

Offset	1	2	3
Time	98	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

Dial 3/Split 3

Phase	1	2	3	4	5	6	7	8
Time	20	101	37	22	20	101	0	0
Mode	0 - AP	1 - CP	0 - AP	0 - AP	0 - AP	1 - CP	0 - AP	0 - AP
Min Veh Time	13	18	12	12	13	18		7
Min Ped Time	0	30	0	0	0	30		

Phase	9	10	11	12	13	14	15	16
Time	0	0	0	0	0	0	0	0
Mode	0 - AP							
Min Veh Time				*****				
Min Ped Time								

Offset	1	2	3
Time	106	0	0
Mode	0 - Normal	0 - Normal	0 - Normal
Alt Sequence	0	0	0
Ring 2 Lag Time	0	0	0
Ring 3 Lag Time	0	0	0
Ring 4 Lag Time	0	0	0

LOCATION OF SIGNAL: S. STATE ST & WEBBS LANE

DATE OF CHART:

8/22/2005

DATE INSTALLED:

PHASES

3/31/2006

CONTROLLER TYPE: EPAC COORDINATION TYPE: ACTRA

COORDINATION TYPE: ACCORDINATION ADDRESS: 10



CHART REVISION: A.1

CABINET TYPE: PCOM MONITOR TYPE: NEMA+

PHASE NUMBER	1	2	3	4	5	6	7	18
PHASE LOCATION	SB LT	NB STATE	WB	EB	NBLT	SB STATE	0 '	0
MIN GRN	5	15	5	5	5	1/5	0	0
PASS/10	30	40	40	40	30	40	0	0
MAX #1	20	60	30	30	20	60	0	0
MAX #2	20	60	30	30	20	60	. 0	0
YEL/10	30	40	40	40	30	40	0	0
RED/10	20	20	20	20	20	20	0 ,	0
A NAME OF THE PROPERTY OF THE			Samuel and American	por service visit de la constitución de la constitu				0
AINI/10	0	20	0	0:	0	20	0	0
MAX INI	0	30	0	0	0	30 0	0	0 0
TIM BEF	0	0	0	0	0	0	0	0
CAR BEF	0	0	0	0	0	. 0	0	0
TIME TO	0	0	0	0	0	0	0	0
MGAP/10	0	U	U	U	0	0		
WALK	0	0	0	0	0	0	0	0
PED CLR	0	0	0	0	0	0	0	0
EXT PCL	0	0	0	0	0	0	0	0
						National Section Methods (1930)	Patro / Supering Property	SCHOOL CONTRACTOR
INITIAL	1	4	1	1	1,	4	0	0
NA RESP	0	0	0	0	0	0	0	0
V. RECALL	0	2	0	0	0	2	0	0
P. RECALL	0	0	0	0	0	0	0	0
NLMEM.	13	0	0	0		0	0	0
2 ENTRY	0	1	0 ,	0	0	1	0	0
A REAL PROPERTY COME AT THE CASE AND RESERVED TO THE RESERVED AND ADDRESS OF THE RESERVED AT THE RESERVED AND ADDRESS OF THE RESERVED ADDRESS OF THE RESERVED AND ADDRESS OF THE RESERVED ADDRESS OF THE R								Tomoreau region comen
OVERLAP/ LOCATION	A=		B=	50.25.7	C=	Sec. Comme	D=	No. of the

** INTERSECTION NOTES **

1 PHASES 1 8	§ 5 ARE PERMISSIVE LEFT TURNS (5-SECTION)	
2 PREEMPT:	TRANS (PRIORITY) = NB + SB , EB, OR WB EMERG (PREEMPT) = SB + SBLT, NB + NBLT, EB OR WB	
3	EMICKS (FILEIMFT) = 30 + 3001, NO + NOCT, ED 31 VVD	·
4		
5		
6		

LOCATION OF SIGNAL: N. STATE ST & GOVENORS AVE

DATE OF CHART: 1/22/02
DATE INSTALLED: 10/1/57
CONTROLLER TYPE: EPAC
COORDINATION TYPE: ACTRA
COORDINATION ADDRESS: 25



CHART REVISION: 2
CABINET TYPE: PCOM
MONITOR TYPE: NEMA+

		*						* *
PHASE NUMBER	1	2	3	4	5	6	7	,8
PHASE LOCATION	SB to State St	NB GOVENOR	WALKER RD	NBLT STATE		SB to Govr's	- 建物建筑	
MIN GRN	5	10	10	5		10		
PASS/10	50	50	40	50	Property of	50	17位接触	1. 水,特种东
MAX #1	99	99	60	30		99		
MAX #2	99	99	60	30	温频探发等程数	99	当的說聲問題	第分的 专家的特别
YEL/10	40	40	40	40		50		
RED/10	20	20	20	20		70		
								port to the second second
AINI/10	水黄 O 表示語	特殊(0)	0/#	0	2000年 (1995年)	10.00	不可以在特別技能的	
MAX INI	0	0	0	0		0	MANAGEM WILLIAM CO. CONTROL	
TIM BEF	0	0	0	0.	trong to the same	0	在企業的基本的	基础
CAR BEF	0	0	0	0		0		· · · · · · · · · · · · · · · · · · ·
TIME TO	0	0	0	0	10004045006	0		6.18.016
MGAP/10	0	0	0	0		0		
					Park Mark Street			10 TO
WALK	學職0	0	0	0	A CONTRACTOR	0	E-W	Law east
PED CLR	0	0	0	0	Electric Corp.	0	W.	
EXT PCL	0	0	0	星秋 0、第		ARREST UNDER STATE OF	ENDE	45099
West-Control		71000	and the second		0	4	0	11 × 0
INITIAL	1223	4	1505	0	THE PERSON NAMED IN	0	である。 は は の の の の の の の の の の の の の	U
NA RESP	0	0	0	U		U		
W SEOWL		2	0	0 1		2	And And Street Show	
V. RECALL	0 10 10 10 10 10 10 10 10 10 10 10 10 10	0	0	0	Maria di Maria di Maria	0	《经验》	
P. RECALL	0	0	U	U		U		
NI MEM	0	0	0.40	0		0		超级能
NL MEM. 2 ENTRY	0	1	0	0		1		274.00
ZENIKI	U	'		U			-	
OVERLAP/ LOCATION	A= hall and				C=	iii maa	D=	ARTES OF A
PHASES		(1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.	· Artista de la constantida del constantida del constantida de la constantida de la constantida de la constantida del constantida de la constantida de la constantida de la constantida del constant		Archete administration	vesta a vesta de la	e de la company	
LIVOEO								

** INTERSECTION NOTES **

1	NB STATE ST. IS FLASHING YELLOW
2	PRIORITIES: SB to State & SB to Govr's or NB & SB to Govr's or EBRT & EBLT or NBLT
3	PRE-EMPTS: SB to State & SB to Govr's or NB & SB to Govr's or EBRT & EBLT or NBLT
4	
5	
6	

Dover Pedestrian Signals Feasibility Study

Appendix C

Critical Movement Summation (CMS)

Analysis

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: Existing Conditions (No Ped Signals)

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	185	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	1092	2	32	569
	6 SB	678		0	373
	3 WBLT	0	0	0	0
	7 EBLT	0	0	0	0
	4 EB	94	2	0	52
	8 WB	54	2	0	30
	1 SBLT	29	1	0	29
	5 NBLT	61	1	0	61
				TOTAL	682

TOTAL 68
SERVICE LEVEL A

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40 CRITICAL MOVEMENT
NB *
SB
WBLT
EBLT
EB *
WB
SBLT
NBLT *

LOCATION: US 13 & Roosevelt Ave ALTERNATIVE: Existing Conditions (No Ped Signals)

TIME OF DAY: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP DATE: 12/16/2014

CYCLE LENGTH: CYCLES/HR: 80

		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED FOR	WALK + DONT		ACTUAL RING	ACTUAL RING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	1 SPLIT	2 SPLIT
2	NB	569	8	20.5	20.5		59%	35.2	
6	SB	0	0	0.0	0.0		0%		26.4
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	52	1	3.8	3.8		22%	9.8	
8	WB	0	0	0.0	0.0		0%		9.8
1	SBLT	0	0	0.0	0.0		0%	0.0	
5	NBLT	61	1	3.8	3.8		20%		8.8
			TOTAL GREEN	28.1			100%	45	45

GREEN
TOTAL
CLEARANCE
(RED +
YELLOW) 17.0 TOTAL TIME REQUIRED 45.1

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: Existing Conditions (No Ped Signals)

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	185	

MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
2 NB	925	2	0	509
6 SB	1247	2	26	66
3 WBLT	0	0	0	
7 EBLT	0	0	0	
4 EB	118	2	0	6
8 WB	47	2	0	20
1 SBLT	46	1	0	4
5 NBLT	20	1	0	20
	2 NB 6 SB 3 WBLT 7 EBLT 4 EB 8 WB 1 SBLT	2 NB 925 6 SB 1247 3 WBLT 0 7 EBLT 0 4 EB 118 8 WB 47 1 SBLT 46	2 NB 925 6 SB 1247 3 WBLT 0 7 EBLT 0 4 EB 118 2 WB 47 2 SBLT 46 1 SBLT 46	2 NB 925 2 0 6 SB 1247 2 26 3 WBLT 0 0 0 7 EBLT 0 0 0 4 EB 118 2 0 8 WB 47 2 0 1 SBLT 46 1 0

TOTAL 777

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40 CRITICAL MOVEMENT
NB
SB *
WBLT
EBLT
EB *
WB

SBLT *

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: Existing Conditions (No Ped Signals)

TIME OF DAY: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

CYCLE LENGTH: 50 CYCLES/HR: 72

PHASES	MOVEMENT	CRITICAL LANE VOLUME	VEHICLES PER CYCLE	GREEN TIME REQUIRED FOR CYCLE	GREEN TIME REQUIRED FOR PHASE	WALK + DONT WALK	%	ACTUAL RING 1 SPLIT	ACTUAL RING 2 SPLIT
2	NB	0	0	0.0	0.0		0%	31.1	
6	SB	660	10	24.7	24.7		62%		40.0
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	65	1	3.8	3.8		20%	9.9	
8	WB	0	0	0.0	0.0		0%		9.9
1	SBLT	46	1	3.8	3.8		18%	8.9	
5	NBLT	0	0	0.0	0.0		0%		0.0
			TOTAL GREEN	32.3			100%	50	50

TOTAL
CLEARANCE
(RED +
YELLOW)
TOTAL TIME
REQUIRED
49.3

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: One-Stage Pedestrian Crossing w/ Concurrent E-W Phasing

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	18.5	

PHASE		MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2	NB	1092	2	32	569
		SB	678		0	
		WBLT	0	0	0	0
		EBLT	0	0	0	0
	4	EB	2800	2	0	1540
	8	WB	54	2	0	30
	1	SBLT	29	1	0	29
	5	NBLT	61	1	0	61
					TOTAL	2170

SERVICE LEVEL

F

SERVICE LEVEL LANE USE FACTOR **CRITICAL MOVEMENT** A - LESS THAN 1,000 V/HR 1 LANE = 1.00 NB * 2 LANES = .55 SB B - 1,001 TO 1,150 V/HR C - 1,151 TO 1,300 V/HR 3 LANES = .40**WBLT** D - 1,301 TO 1,450 V/HR **EBLT** E - 1,451 TO 1,600 V/HR EB * F - MORE THAN 1,600 V/HR WB SBLT **NBLT** *

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: One-Stage Pedestrian Crossing w/ Concurrent E-W Phasing

TIME OF DAY: AM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 45 CYCLES/HR: 80

PHASES	MOVEMENT	CRITICAL LANE VOLUME	VEHICLES PER CYCLE	GREEN TIME REQUIRED FOR CYCLE	GREEN TIME REQUIRED FOR PHASE	WALK + DONT WALK	%	ACTUAL RING 1 SPLIT	ACTUAL RING 2 SPLIT
2	NB	569	8	20.5	20.5	13	30%	18.3	
6	SB	0	0	0.0	0.0		0%		13.7
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	1540	20	45.7	45.7	45	59%	26.7	
8	WB	0	0	0.0	0.0		0%		26.7
1	SBLT	0	0	0.0	0.0		0%	0.0	
5	NBLT	61	1	3.8	3.8		10%		4.6
	•	•	TOTAL GREEN	70.0			100%	45	45

TOTAL CLEARANCE (RED + YELLOW) 17.0

TOTAL TIME REQUIRED 87.0

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: One-Stage Pedestrian Crossing w/ Concurrent E-W Phasing

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	185	

PHASE		MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2	NB	925	2	0	509
	6	SB	1247	2	26	660
	3	WBLT	0	0	0	0
	7	EBLT	0	0	0	0
	4	ЕВ	2500	2	0	1375
	8	WB	47	2	0	26
	1	SBLT	46	1	0	46
	5	NBLT	20	1	0	20
					TOTAL	2004

TOTAL 2081
SERVICE LEVEL F

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40

NB
SB *
WBLT
EBLT
EB *
WB
SBLT *
NBLT

CRITICAL MOVEMENT

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: One-Stage Pedestrian Crossing w/ Concurrent E-W Phasing

TIME OF DAY: PM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 50 CYCLES/HR: 72

		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED FOR	WALK + DONT		ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	RING 1 SPLIT	RING 2 SPLI
2	NB	0	0	0.0	0.0		0%	16.8	
6	SB	660	10	24.7	24.7	13	34%		21.6
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	1375	20	45.7	45.7	45	57%	28.3	
8	WB	0	0	0.0	0.0		0%		28.3
1	SBLT	46	1	3.8	3.8		10%	4.8	
5	NBLT	0	0	0.0	0.0		0%		0.0
	•	•	TOTAL GREEN	74.2			100%	50	50

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED
91.2

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: Two-Stage Crossing with Exclusive Pedestrian Phase

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	185	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
:	2 NB	1092	2	32	569
	SB	678	2	0	373
;	WBLT	0	0	0	O
	PEDS	600	1	0	600
	4 EB	2150	2	600	583
	B WB	54	2	0	30
	1 SBLT	29	1	0	29
· ·	NBLT	61	1	0	61
				TOTAL	1813

SERVICE LEVEL

SERVICE LEVEL LANE USE FACTOR **CRITICAL MOVEMENT** A - LESS THAN 1,000 V/HR 1 LANE = 1.00 NB * B - 1,001 TO 1,150 V/HR 2 LANES = .55 SB C - 1,151 TO 1,300 V/HR 3 LANES = .40**WBLT** D - 1,301 TO 1,450 V/HR PEDS * EB * E - 1,451 TO 1,600 V/HR F - MORE THAN 1,600 V/HR WB SBLT NBLT *

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: Two-Stage Crossing with Exclusive Pedestrian Phase

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

CYCLE LENGTH: 45 CYCLES/HR: 80

		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED FOR	WALK + DONT		ACTUAL RING	ACTUAL RING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	1 SPLIT	2 SPLIT
2	NB	569	8	20.5	20.5	13	32%	19.3	
6	SB	0	0	0.0	0.0		0%		14.5
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	PEDS	600	8	20.5	20.5	20	25%		11.2
4	EB	583	8	20.5	20.5	20	32%	25.7	
8	WB	0	0	0.0	0.0		0%		14.5
1	SBLT	0	0	0.0	0.0	•	0%	0.0	
5	NBLT	61	1	3.8	3.8		11%		4.8
			TOTAL GREEN	65.3			100%	45	45

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

82.3

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: Two-Stage Crossing with Exclusive Pedestrian Phase

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

286	3&8	487	1&5	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	925	2	0	509
	6 SB	1247		26	
	3 WBLT	0			
	7 PEDS	550		0	550
	4 EB	1950		550	
	8 WB	47		0	26
	1 SBLT	46	1	0	46
	5 NBLT	20	1	0	20
				TOTAL	1779

SERVICE LEVEL

F

SERVICE LEVEL LANE USE FACTOR **CRITICAL MOVEMENT** A - LESS THAN 1,000 V/HR 1 LANE = 1.00 NB B - 1,001 TO 1,150 V/HR 2 LANES = .55 SB * C - 1,151 TO 1,300 V/HR 3 LANES = .40**WBLT** D - 1,301 TO 1,450 V/HR PEDS * EB * E - 1,451 TO 1,600 V/HR F - MORE THAN 1,600 V/HR WB SBLT * **NBLT**

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: Two-Stage Crossing with Exclusive Pedestrian Phase

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

CYCLE LENGTH: 50 CYCLES/HR: 72

				GREEN TIME	GREEN TIME	WALK +			
		CRITICAL LANE	VEHICLES	REQUIRED FOR	REQUIRED FOR	DONT		ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	RING 1 SPLIT	RING 2 SPLIT
2	NB	0	0	0.0	0.0		0%	17.7	
6	SB	660	10	24.7	24.7	13	35%		22.8
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	PEDS	550	8	20.5	20.5	20	24%		11.8
4	EB	523	8	20.5	20.5	20	31%	27.1	
8	WB	0	0	0.0	0.0		0%		15.3
1	SBLT	46	1	3.8	3.8		10%	5.1	
5	NBLT	0	0	0.0	0.0		0%		0.0
			TOTAL GREEN	69.5			100%	50	50

TOTAL CLEARANCE (RED + YELLOW) 17.0

TOTAL TIME REQUIRED 86.5

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: 2-Stage Pedestrian Crossing with E-W Split Phase

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	185	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	1092	2	2 32	569
	6 SB	678	2	2 0	373
	3 WBLT	0	C	0	0
	7 EBLT	0	(0	0
	4 EB	709	2	2 0	390
	8 WB	710	2	2 0	391
	1 SBLT	29	1	0	29
	5 NBLT	61	1	0	61
				TOTAL	1411

SERVICE LEVEL

D

SERVICE LEVEL A - LESS THAN 1,000 V/HR B - 1,001 TO 1,150 V/HR C - 1,151 TO 1,300 V/HR	LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40	CRITICAL MOVEMENT NB * SB WBLT
D - 1,301 TO 1,450 V/HR E - 1,451 TO 1,600 V/HR		EBLT EB *
F - MORE THAN 1,600 V/HR		WB * SBLT NBLT *
		INDLI

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: 2-Stage Pedestrian Crossing with E-W Split Phase

CYCLES/HR: 55

HOUR: AM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH:

65

				GREEN TIME	GREEN TIME	WALK +			
		CRITICAL LANE	VEHICLES	REQUIRED FOR	REQUIRED FOR	DONT			ACTUAL RING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	1 SPLIT	2 SPLIT
2	NB	569	11	26.8	26.8	13	34%	23.8	
6	SB	0	0	0.0	0.0		0%		21.8
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	390	8	20.5	20.5	20	27%	17.6	
8	WB	391	8	20.5	20.5	20	27%		17.6
1	SBLT	29	1	0.0	3.8		0%	5.9	
5	NBLT	61	2	6.9	6.9		12%		7.9
			TOTAL	74.7			100%	47	47

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: 2-Stage Pedestrian Crossing with E-W Split Phase

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	1&5	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	925	2	0	509
	6 SB	1247		26	
	3 WBLT	0	0	0	C
	7 EBLT	0	0	0	O
	4 EB	674	2	0	371
	8 WB	675	2	0	372
	1 SBLT	46	1	0	46
	5 NBLT	20	1	0	20

TOTAL 1449
SERVICE LEVEL D

SERVICE LEVEL	LANE USE FACTOR
A - LESS THAN 1,000 V/HR	1 LANE = 1.00
B - 1,001 TO 1,150 V/HR	2 LANES = .55
C - 1,151 TO 1,300 V/HR	3 LANES = .40
D - 1,301 TO 1,450 V/HR	
E - 1,451 TO 1,600 V/HR	
F - MORE THAN 1,600 V/HR	

CRITICAL MOVEMENT
NB
SB *
WBLT
EBLT
EB *
WB *
SBLT *
NBLT

LOCATION: US 13 & Roosevelt Ave

ALTERNATIVE: 2-Stage Pedestrian Crossing with E-W Split Phase

HOUR: PM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 68 CYCLES/HR: 53

				GREEN TIME	GREEN TIME	WALK +			
		CRITICAL LANE	VEHICLES	REQUIRED FOR	REQUIRED FOR	DONT		ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	RING 1 SPLIT	RING 2 SPLIT
2	NB	0	0	0.0	0.0		0%	25.5	
6	SB	660	13	31.0	31.0	13	37%		25.5
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	371	8	20.5	20.5	20	27%	18.2	
8	WB	372	8	20.5	20.5	20	27%		18.2
1	SBLT	46	1	0.0	3.8		4%	2.6	
5	NBLT	20	1	3.8	3.8		0%		2.6
			TOTAL GREEN	75.8			95%	46	46

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

98.8

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

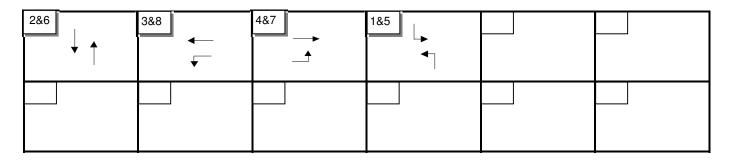
LOCATION: US Route 13 & Webbs Lane

ALTERNATIVE: Existing Conditions (No Ped Signals)

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	1296	2	172	541
	6 SB	799	2	0	440
	3 WBLT	0	0	0	0
	7 EBLT	0	0	0	0
	4 EB	243	2	0	134
	B WB	169	2	0	93
	SBLT	7	1	0	7
	5 NBLT	179	1	0	179

TOTAL 947
SERVICE LEVEL A

SERVICE LEVEL	LANE USE FACTOR
A - LESS THAN 1,000 V/HR	1 LANE = 1.00
B - 1,001 TO 1,150 V/HR	2 LANES = .55
C - 1,151 TO 1,300 V/HR	3 LANES = .40
D - 1,301 TO 1,450 V/HR	
E - 1,451 TO 1,600 V/HR	
F - MORE THAN 1,600 V/HR	

CRITICAL MOVEMENT
NB *
SB
WBLT
EBLT
EB *
WB *
SBLT
NBLT *

LOCATION: US Route 13 & Webbs Lane

ALTERNATIVE: Existing Conditions (No Ped Signals)

HOUR: AM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

271121 127 107 201 1

CYCLE LENGTH: 90 CYCLES/HR: 40

PHASES	MOVEMENT	CRITICAL LANE VOLUME	VEHICLES PER CYCLE	GREEN TIME REQUIRED FOR CYCLE	GREEN TIME REQUIRED FOR PHASE	WALK + DONT WALK	%	ACTUAL RING 1 SPLIT	ACTUAL RING 2 SPLIT
2	NB	541	14	33.1	33.1		43%	48.5	
6	SB	0	0	0.0	0.0		0%		38.3
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	134	4	12.0	12.0		20%	17.6	
8	WB	93	3	9.6	9.6		17%		17.6
1	SBLT	7	1	0.0	3.8		0%	8.6	
5	NBLT	179	5	14.2	14.2		21%		18.8
			TOTAL GREEN	68.9			100%	75	75

GREEN	08.9			
TOTAL		CLEARANCE	(RED + YELLOW)	23.0
TOTAL TIME	REQUIRED	91.9		

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

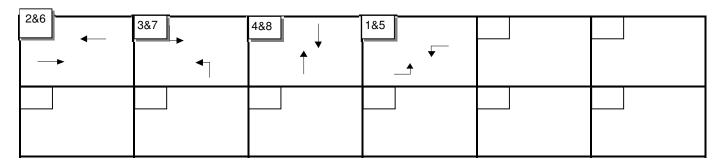
LOCATION: US Route 13 & Webbs Lane

ALTERNATIVE: Existing Conditions (No Ped Signals)

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	1100	2	93	512
	S SB	1494		0	822
	3 WBLT	0		0	
	7 EBLT	0	0	0	0
	1 EB	344	2	0	190
	3 WB	196	2	0	108
	1 SBLT	16	1	0	16
	5 NBLT	109	1	0	109

TOTAL 1229
SERVICE LEVEL C

SERVICE LEVEL	LANE USE FACTOR
A - LESS THAN 1,000 V/HR	1 LANE = 1.00
B - 1,001 TO 1,150 V/HR	2 LANES = .55
C - 1,151 TO 1,300 V/HR	3 LANES = .40
D - 1,301 TO 1,450 V/HR	
E - 1,451 TO 1,600 V/HR	
F - MORE THAN 1,600 V/HR	

CRITICAL MOVEMENT
NB
SB *
WBLT
EBLT
EB *
WB *
SBLT
NBLT *

LOCATION: US 13 & Webbs Lane

ALTERNATIVE: Existing Conditions (No Ped Signals)

HOUR: PM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 130 CYCLES/HR: 28

		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED FOR	WALK + DONT		ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%		RING 2 SPLIT
2	NB	0	0	0.0	0.0		0%	79.5	
6	SB	822	30	66.7	66.7		55%		71.5
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	190	7	18.4	18.4		18%	24.0	
8	WB	108	4	12.0	12.0		14%		24.0
1	SBLT	16	1	0.0	3.8		0%	8.7	
5	NBLT	109	4	12.0	12.0		13%		16.7
			TOTAL GREEN	109.1			100%	112	112

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

132.1

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Webbs Lane

ALTERNATIVE: 1-Stage Pedestrian Crossing

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	4&7	185	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	1296	2	172	541
	6 SB	799			
	3 WBLT	o		0	
	7 EBLT	0	O	0	0
	4 EB	1050	2	2 0	578
	8 WB	169	2	2 0	93
	1 SBLT	7	1	0	7
	5 NBLT	179	1	0	179
				TOTAL	1391

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40 CRITICAL MOVEMENT

NB *

SB

WBLT

EBLT

EB *

WB *

SBLT

NBLT *

D

SERVICE LEVEL

LOCATION: US 13 & Webbs Lane **ALTERNATIVE: 1-Stage Pedestrian Crossing**

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP DATE: 12/16/2014

CYCLE LENGTH: 90 CYCLES/HR: 40

		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED FOR	WALK + DONT		ACTUAL RING	ACTUAL RING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	1 SPLIT	2 SPLIT
2	NB	541	14	33.1	33.1	16	34%	38.7	
6	SB	0	0	0.0	0.0		0%		30.6
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	578	15	35.2	35.2	35	36%	32.2	
8	WB	93	3	9.6	9.6		14%		32.2
1	SBLT	7	1	0.0	3.8		0%	6.9	
5	NBLT	179	5	14.2	14.2	•	17%		15.0
		_	TOTAL GREEN	92.1		•	100%	78	78

TOTAL CLEARANCE (RED + YELLOW) 23.0 TOTAL TIME REQUIRED 115.1

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Webbs Lane

ALTERNATIVE: 1-Stage Pedestrian Crossing

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

286	3&8	487	185	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
2	NB	1100	2	93	512
	SB	1494	2		
	WBLT	0	0	0	
	'EBLT	0	0	0	0
4	EB	750	2	0	413
8	w B	196	2	0	108
1	SBLT	16	1	0	16
5	NBLT	109	1	0	109
				TOTAL	1452

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40

NB SB * WBLT EBLT EB *

CRITICAL MOVEMENT

SERVICE LEVEL

WB * SBLT NBLT *

LOCATION: US 13 & Webbs Lane

ALTERNATIVE: 1-Stage Pedestrian Crossing

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

CYCLE LENGTH: 130 CYCLES/HR: 28

		1	1	ODEEN TIME	ODEEN TIME	14/41/4			
		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED	WALK + DONT		ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	FOR PHASE	WALK	%		RING 2 SPLIT
	_					WALK			NING 2 SPLIT
2	NB	0	0	0.0	0.0		0%	70.6	
6	SB	822	30	66.7	66.7	16	49%		63.5
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	413	15	35.2	35.2	35	28%	36.0	
8	WB	108	4	12.0	12.0		12%		36.0
1	SBLT	16	1	0.0	3.8		0%	7.7	
5	NBLT	109	4	12.0	12.0		11%		14.8
			TOTAL GREEN	125.9			100%	114	114

TOTAL CLEARANCE (RED + YELLOW) 23.0

TOTAL TIME REQUIRED 148.9

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Webbs Lane

ALTERNATIVE: 2-Stage Pedestrian Crossing

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

286	3&8	487	185	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	1296	2	172	541
	6 SB	799		0	440
	3 WBLT	0	0	0	0
	7 EBLT	0	0	0	0
	4 EB	550	2	0	303
	8 WB	400	2	0	220
	1 SBLT	7	1	0	7
	5 NBLT	179	1	0	179
				TOTAL	1243

SERVICE LEVEL LANE USE FACTOR **CRITICAL MOVEMENT** A - LESS THAN 1,000 V/HR 1 LANE = 1.00 B - 1,001 TO 1,150 V/HR 2 LANES = .55 C - 1,151 TO 1,300 V/HR 3 LANES = .40 D - 1,301 TO 1,450 V/HR E - 1,451 TO 1,600 V/HR F - MORE THAN 1,600 V/HR

NB * SB **WBLT EBLT** EB * WB * SBLT NBLT *

SERVICE LEVEL

LOCATION: US 13 & Webbs Lane ALTERNATIVE: 2-Stage Pedestrian Crossing TIME OF DAY: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP DATE: 12/16/2014

CYCLE LENGTH: CYCLES/HR: 40

		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED FOR	WALK + DONT		ACTUAL RING	ACTUAL RING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	1 SPLIT	2 SPLIT
2	NB	541	14	33.1	33.1	16	37%	41.6	
6	SB	0	0	0.0	0.0		0%		32.9
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	303	8	20.5	20.5	20	25%	22.3	
8	WB	220	6	16.3	16.3	16	21%		22.3
1	SBLT	7	1	0.0	3.8		0%	7.4	
5	NBLT	179	5	14.2	14.2		18%		16.1
			TOTAL GREEN	84.1			100%	71	71

TOTAL CLEARANCE (RED + 23.0 YELLOW) TOTAL TIME REQUIRED 107.1

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: US 13 & Webbs Lane

ALTERNATIVE: 2-Stage Pedestrian Crossing

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	3&8	487	185	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	1100	2	93	512
		1100		33	312
	6 SB	1494	2	0	822
	3 WBLT	0	0	0	0
	7 EBLT	0	0	0	0
	4 EB	400	2	0	220
	8 WB	300	2	0	165
	1 SBLT	16	1	0	16
	5 NBLT	109	1	0	109
				TOTAL	1216

TOTAL 1316

SERVICE LEVEL D

ERVICE LEVEL LANE USE FACTOR CRITICAL MOVEMENT

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40

NB
SB *
WBLT
EBLT
EB *
WB *
SBLT
NBLT *

LOCATION: US 13 & Webbs Lane

ALTERNATIVE: 2-Stage Pedestrian Crossing

TIME OF DAY: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

CYCLE LENGTH: 130 CYCLES/HR: 28

				GREEN TIME	GREEN TIME	WALK +			
		CRITICAL LANE		REQUIRED FOR	REQUIRED	DONT		ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	FOR PHASE	WALK	%	RING 1 SPLIT	RING 2 SPLIT
2	NB	0	0	0.0	0.0		0%	75.9	
6	SB	822	30	66.7	66.7	16	52 %		68.2
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	220	8	20.5	20.5	20	19%	24.9	
8	WB	165	6	16.3	16.3	16	16%		24.9
1	SBLT	16	1	0.0	3.8		0%	8.3	
5	NBLT	109	4	12.0	12.0		12%		16.0
			TOTAL GREEN	115.5			100%	109	109

TOTAL CLEARANCE (RED + YELLOW) 23.0

TOTAL TIME REQUIRED 138.5

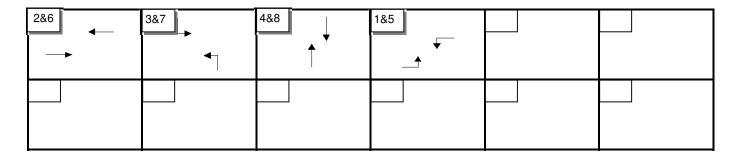
CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: South State St & Webbs Lane ALTERNATIVE: Existing (No Ped Signals)

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
2	NB	806	1	97	709
	SB	274		0	274
	WBLT	0		0	0
	EBLT	0	_	0	0
	EB	48	1	0	48
	WB	64		0	64
1	SBLT	36		0	36
5	NBLT	133		0	133

TOTAL 954
SERVICE LEVEL A

SERVICE LEVEL	LANE USE FACTOR
A - LESS THAN 1,000 V/HR	1 LANE = 1.00
B - 1,001 TO 1,150 V/HR	2 LANES = .55
C - 1,151 TO 1,300 V/HR	3 LANES = .40
D - 1,301 TO 1,450 V/HR	
E - 1,451 TO 1,600 V/HR	
F - MORE THAN 1,600 V/HR	

LOCATION: South State St & Webbs Lane
ALTERNATIVE: Existing Conditions (No Ped Signals)
HOUR: AM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 90 CYCLES/HR: 40

CYCLE LENGTH:	90	CYCLES/HR:	40							
PHASES	MOVEMENT	CRITICAL LANE VOLUME	VEHICLES PER CYCLE	GREEN TIME REQUIRED FOR CYCLE	REQUIRED FOR PHASE	COMMENTS	WALK + DONT WALK	%	ACTUAL RING 1 SPLIT	ACTUAL RING 2 SPLIT
2	NB	709	18	41.5	41.5			53%	55.4	
6	SB	0	0	0.0	0.0			0%		47.3
3	WBLT	0	0	0.0	0.0			0%	0.0	
7	EBLT	0	0	0.0	0.0			0%		0.0
4	EB	48	2	6.9	6.9			14%	12.9	
8	WB	64	2	6.9	6.9			14%		12.9
1	SBLT	36	1	0.0	3.8			0%	8.8	
5	NBLT	133	4	12.0	12.0			19%		16.9
		_	TOTAL GREEN	67.3				100%	77	77
			TOTAL CLEARANCE (RED + YELLOW)	23.0					•	
			TOTAL TIME							

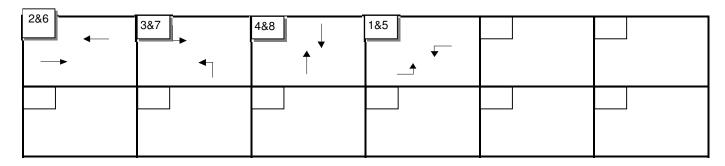
REQUIRED 90.3 CYCLE LENGTH CYCLES/HR 45 80 60 48 40 36 30 24 20 60 75 90 100 120 150 180 PHASE YELLOW TOTAL MIN GRN 2.0 2.0 0.0 0.0 2.0 2.0 2.0 2.0 4.0 4.0 0.0 0.0 4.0 4.0 3.0 3.0 2 6 3 7 4 8 6 0 0 6 6 0 5 7 7 0 7 0 5 5

LOCATION: South State St & Webbs Lane ALTERNATIVE: Existing (No Ped Signals)

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	493	1	105	388
	S SB	911		0	911
	3 WBLT	0		0	
	7 EBLT	0	0	0	0
	1 EB	39	1	0	39
	3 WB	98	1	0	98
	1 SBLT	30	1	0	30
	5 NBLT	135	1	0	135

TOTAL 1183
SERVICE LEVEL C

SERVICE LEVEL	LANE USE FACTOR
A - LESS THAN 1,000 V/HR	1 LANE = 1.00
B - 1,001 TO 1,150 V/HR	2 LANES = .55
C - 1,151 TO 1,300 V/HR	3 LANES = .40
D - 1,301 TO 1,450 V/HR	
E - 1,451 TO 1,600 V/HR	
F - MORE THAN 1,600 V/HR	

CRITICAL MOVEMENT
NB
SB *
WBLT
EBLT
EB *
WB *
SBLT
NBLT *

LOCATION: South State St & Webbs Lane
ALTERNATIVE: Existing Conditions (No Ped Signals)
HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP DATE: 12/16/2014

CYCLE LENGTH: 130 CYCLES/HR: 28

PHASES	MOVEMENT	CRITICAL LANE	VEHICLES PER CYCLE	GREEN TIME REQUIRED FOR CYCLE	GREEN TIME REQUIRED FOR PHASE	COMMENTS	WALK + DONT WALK	%	ACTUAL RING 1 SPLIT	ACTUAL RING 2 SPLIT
2	NB	0	0	0.0	0.0			0%	86.9	
6	SB	911	33	73.0	73.0			61%		79.6
3	WBLT	0	0	0.0	0.0			0%	0.0	
7	EBLT	0	0	0.0	0.0			0%		0.0
4	EB	39	2	6.9	6.9			10%	18.1	
8	WB	98	4	12.0	12.0			14%		18.1
1	SBLT	30	2	0.0	6.9			0%	12.0	
5	NBLT	135	5	14.2	14.2			15%		19.3
			TOTAL GREEN	106.1				100%	117	117
			TOTAL CLEARANCE (RED + YELLOW)	23.0						
CLE LENGTH	CYCLES/HR		TOTAL TIME REQUIRED	129.1						

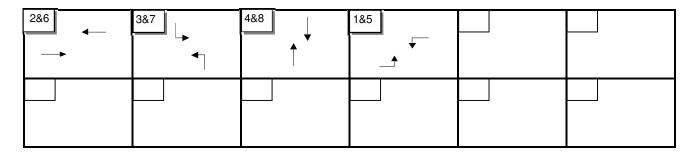
CYCLE LENGTH	CYCLES/HR		REQUIRED	129.1			
	45	80			-		
	60	60					
	75	48	PHASE	YELLOW	RED	TOTAL	MIN GRN
9	90	40	2	4.0	2.0	0	7
10	00	36	6	4.0	2.0	6	7
1:	20	30	3	0.0	0.0	0	7
1:	50	24	7	0.0	0.0	0	0
18	80	20	4	4.0	2.0	6	7
			8	4.0	2.0	6	0
			1	3.0	2.0	0	5
			5	3.0	2.0	5	5

LOCATION: South State St & Webbs Lane ALTERNATIVE: 1-Stage Pedestrian Crossing

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE		MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2	NB	806	1	97	709
	6	SB	274	1	0	274
	3	WBLT	0	0	0	
	7	EBLT	0	0	0	
	4	ЕВ	48	1	0	48
	8	WB	285	1	0	28
	1	SBLT	36	1	0	30
	5	NBLT	133	1	0	13:
					0 0	

TOTAL 1175
SERVICE LEVEL C

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR
1 LANE = 1.00
2 LANES = .55
3 LANES = .40

CRITICAL MOVEMENT
NB *
SB
WBLT
EBLT
EB *
WB *
SBLT
NBLT *

LOCATION: South State St & Webbs Lane
ALTERNATIVE: With Pedestrian Signals
HOUR: AM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 90 CYCLES/HR: 40

		I	1	GREEN TIME	GREEN TIME	WALK +	ı	T.	1
		CRITICAL LANE	VEHICLES	REQUIRED FOR	REQUIRED FOR	DONT		ACTUAL RING	ACTUAL RING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	1 SPLIT	2 SPLIT
2	NB	709	18	41.5	41.5	24	46%	48.2	
6	SB	0	0	0.0	0.0		0%		41.1
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	48	2	6.9	6.9		12%	23.0	
8	WB	285	8	20.5	20.5	19	26%		23.0
1	SBLT	36	1	0.0	3.8		0%	7.6	
5	NBLT	133	4	12.0	12.0		16%		14.7
			TOTAL GREEN	80.9		•	100%	79	79

TOTAL CLEARANCE (RED + YELLOW) 23.0

TOTAL TIME REQUIRED 103.9

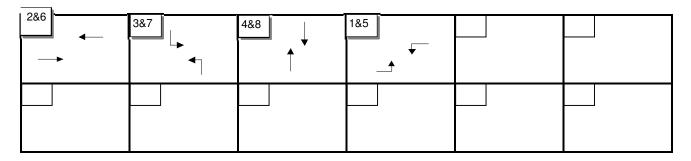
CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

LOCATION: South State St & Webbs Lane ALTERNATIVE: 1-Stage Pedestrian Crossing

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
	2 NB	493	1	105	388
	2 110	733		103	300
	6 SB	911	1	0	911
	3 WBLT	0	0	0	0
	7 EBLT	0	0	0	0
	4 EB	39	1	0	39
	8 WB	200	1	0	200
	1 SBLT	30	1	0	30
	5 NBLT	135	1	0	135
				TOTAL	1295

TOTAL 1285
SERVICE LEVEL C

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40 CRITICAL MOVEMENT
NB
SB *
WBLT
EBLT
EB *
WB *
SBLT
NBLT *

LOCATION: South State St & Webbs Lane ALTERNATIVE: With Pedestrian Signals

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

CYCLE LENGTH: 130 CYCLES/HR: 28

PHASES	MOVEMENT	CRITICAL LANE VOLUME	VEHICLES PER CYCLE	GREEN TIME REQUIRED FOR CYCLE	GREEN TIME REQUIRED FOR PHASE	WALK + DONT WALK	%	ACTUAL RING 1 SPLIT	ACTUAL RING 2 SPLIT
2	NB	0	0	0.0	0.0		0%	81.5	
6	SB	911	33	73.0	73.0	24	57%		74.6
3	WBLT	0	0	0.0	0.0		0%	0.0	
7	EBLT	0	0	0.0	0.0		0%		0.0
4	EB	39	2	6.9	6.9		9%	25.0	
8	WB	200	8	20.5	20.5	19	19%		25.0
1	SBLT	30	2	0.0	6.9		0%	11.2	
5	NBLT	135	5	14.2	14.2		14%		18.1
			TOTAL GREEN	114.6			100%	118	118

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

137.6

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	4.0	2.0	7
75	48	3	0.0	0.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	4.0	2.0	0
150	24	1	3.0	2.0	5
180	20	5	3.0	2.0	5

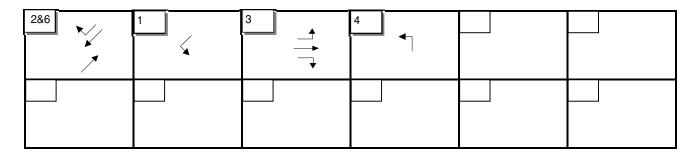
LOCATION: N State St / N Governors Ave & Walker Rd

ALTERNATIVE: Existing (No Ped Signals)

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
2	NB (Governors)	121	1	0	121
	SB (RT)	591	1	0	591
	EB	317	2	0	175
7		0	-	0	0
4	NBLT (State)	105	1	0	105
. 8		0	0	0	0
	SBLT	133	1	0	133
5		0	0	0	0

TOTAL 1004
SERVICE LEVEL B

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40 CRITICAL MOVEMENT
NB (Governors)
SB (RT) *
EB *
0
NBLT (State) *
0
SBLT *
0

LOCATION: N State St / N Governors Ave & Walker Rd ALTERNATIVE: Existing Conditions (No Ped Signals)

TIME OF DAY: AM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 120 CYCLES/HR: 30

				GREEN TIME	GREEN TIME	WALK +			
		CRITICAL LANE	VEHICLES	REQUIRED FOR	REQUIRED FOR	DONT		ACTUAL RING	ACTUAL RING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	WALK	%	1 SPLIT	2 SPLIT
2	NB (Governors)	0	0	0.0	0.0		0%	58.6	
6	SB (RT)	591	20	45.7	45.7		49%		79.1
3	EB	175	6	16.3	16.3		19%	22.6	
7	0	0	0	0.0	0.0		0%		0.0
4	NBLT (State)	105	4	12.0	12.0		15%	18.3	
8	0	0	0	0.0	0.0		0%		40.9
1	SBLT	133	5	14.2	14.2		17%	20.5	
5	0	0	0	0.0	0.0		0%		0.0
			TOTAL GREEN	88.2			100%	120	120

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

118.2

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	5.0	7.0	7
75	48	3	4.0	2.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	0.0	0.0	0
150	24	1	4.0	2.0	5
180	20	5	0.0	0.0	5

LOCATION: N State St / N Governors Ave & Walker Rd

ALTERNATIVE: Existing (No Ped Signals)

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	1	3	4	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
2	NB (Governors)	225	1	0	225
	SB (RT)	478		0	478
	EB	533		0	294
7	,	0	0	0	0
4	NBLT (State)	112	1	0	112
8		0	0	0	0
1	SBLT	132	1	0	132
5		0	0	0	0

TOTAL 1016
SERVICE LEVEL B

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40 CRITICAL MOVEMENT
NB (Governors)
SB (RT) *
EB *
0
NBLT (State) *
0
SBLT *
0

LOCATION: N State St / N Governors Ave & Walker Rd ALTERNATIVE: Existing Conditions (No Ped Signals)

TIME OF DAY: PM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 120 CYCLES/HR: 30

	1	1		ODEEN TIME	ODEEN TIME	14/41.1/		1	
		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED	WALK + DONT		ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	FOR PHASE	WALK	%	RING 1 SPLIT	
2	NB (Governors)	0	0	0.0	0.0		0%	50.1	
6	SB (RT)	478	16	37.3	37.3		42%		70.6
3	ЕВ	294	10	24.7	24.7		26%	31.2	
7	0	0	0	0.0	0.0		0%		0.0
4	NBLT (State)	112	4	12.0	12.0		15%	18.3	
8	0	0	0	0.0	0.0		0%		49.5
1	SBLT	132	5	14.2	14.2		17%	20.5	
5	0	0	0	0.0	0.0		0%		0.0
			TOTAL GREEN	88.2			100%	120	120

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

118.2

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	MIN GRN
45	80	2	4.0	2.0	7
60	60	6	5.0	7.0	7
75	48	3	4.0	2.0	7
90	40	7	0.0	0.0	0
100	36	4	4.0	2.0	7
120	30	8	0.0	0.0	0
150	24	1	4.0	2.0	5
180	20	5	0.0	0.0	5

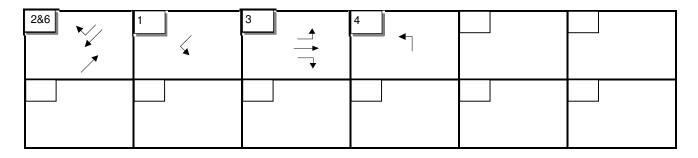
LOCATION: N State St / N Governors Ave & Walker Rd

ALTERNATIVE: With Pedestrian Signals

HOUR: AM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014



PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
2	NB (Governors)	121	1	0	121
	SB (RT)	591	1	0	591
	EB	317		0	175
				0	
	PEDS	190		0	190
	NBLT (State)	125	1	0	125
8		0	0	0	0
1	SBLT	133	1	0	133
5		0	0	0	0

TOTAL 1214 SERVICE LEVEL С

SERVICE LEVEL A - LESS THAN 1,000 V/HR B - 1,001 TO 1,150 V/HR C - 1,151 TO 1,300 V/HR D - 1,301 TO 1,450 V/HR E - 1,451 TO 1,600 V/HR F - MORE THAN 1,600 V/HR LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40

CRITICAL MOVEMENT NB (Governors)

SB (RT) *

EB * PEDS *

NBLT (State) *

0

SBLT *

0

LOCATION: N State St / N Governors Ave & Walker Rd ALTERNATIVE: With Pedestrian Signals

HOUR: AM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 120 CYCLES/HR: 30

		CRITICAL LANE	VEHICLES	GREEN TIME REQUIRED FOR	GREEN TIME REQUIRED FOR		WALK + DONT		ACTUAL RING	ACTUAL DING
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	COMMENTS	WALK	%	1 SPLIT	2 SPLIT
2	NB (Governors)	0	0	0.0	0.0			0%	49.9	
6	SB (RT)	591	20	45.7	45.7			42%		67.4
3	EB	175	6	16.3	16.3			16%	14.1	
7	PEDS	190	7	18.4	18.4		17	13%		15.9
4	NBLT (State)	125	5	14.2	14.2		14	15%	19.3	
8	0	0	0	0.0	0.0			0%		17.5
1	SBLT	133	5	14.2	14.2			15%	17.5	
5	0	0	0	0.0	0.0			0%		0.0
			TOTAL GREEN	108.8				100%	101	101

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

138.8

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	TOTAL	MIN GRN
45	80	2	4.0	2.0	0	7
60	60	6	5.0	7.0	12	7
75	48	3	4.0	2.0	6	7
90	40	7	0.0	0.0	0	0
100	36	4	4.0	2.0	6	7
120	30	8	0.0	0.0	0	0
150	24	1	4.0	2.0	6	5
180	20	5	0.0	0.0	0	5

LOCATION: N State St / N Governors Ave & Walker Rd

ALTERNATIVE: With Pedestrian Signals

HOUR: PM Peak Hour

DONE BY: KOC CHECKED BY: AJP

DATE: 12/16/2014

2&6	1	3	4	

PHASE	MOVEMENT	VOLUME	# of lanes	Left Turn Credit	CLV
2	NB (Governors)	225	1	0	225
	SB (RT)	478		0	478
	EB	533		0	294
	PEDS	255		0	255
	NBLT (State)	200		0	200
8		0	0	0	0
1	SBLT	132	1	0	132
5		0	0	0	0

TOTAL 1359
SERVICE LEVEL D

SERVICE LEVEL
A - LESS THAN 1,000 V/HR
B - 1,001 TO 1,150 V/HR
C - 1,151 TO 1,300 V/HR
D - 1,301 TO 1,450 V/HR
E - 1,451 TO 1,600 V/HR
F - MORE THAN 1,600 V/HR

LANE USE FACTOR 1 LANE = 1.00 2 LANES = .55 3 LANES = .40 CRITICAL MOVEMENT

NB (Governors)

SB (RT) *

EB *

PEDS *

NBLT (State) *

0

SBLT *

0

LOCATION: N State St / N Governors Ave & Walker Rd

ALTERNATIVE: With Pedestrian Signals

HOUR: PM Peak Hour

DONE BY: KOC
CHECKED BY: AJP
DATE: 12/16/2014

CYCLE LENGTH: 85 CYCLES/HR: 42

BUAGEG	MOVEMENT	CRITICAL LANE		REQUIRED FOR	REQUIRED FOR		DONT	0/	ACTUAL	ACTUAL
PHASES	MOVEMENT	VOLUME	PER CYCLE	CYCLE	PHASE	COMMENTS	WALK	%		RING 2 SPLIT
2	(Governo	0	0	0.0	0.0			0%	28.5	
6	SB (RT)	478	12	28.9	28.9			34%		41.1
3	EB	294	7	18.4	18.4			20%	17.0	
7	PEDS	255	7	18.4	18.4		17	15%		17.0
4	BLT (State	200	5	14.2	14.2		14	17%	14.1	
8	0	0	0	0.0	0.0			0%		14.1
1	SBLT	132	4	12.0	12.0			15%	12.6	
5	0	0	0	0.0	0.0			0%		0.0
		·	TOTAL GREEN	91.9		·		100%	72	72

TOTAL
CLEARANCE
(RED +
YELLOW)

TOTAL TIME
REQUIRED

121.9

CYCLE LENGTH	CYCLES/HR	PHASE	YELLOW	RED	TOTAL	MIN GRN
45	80	2	4.0	2.0	0	7
60	60	6	5.0	7.0	12	7
75	48	3	4.0	2.0	6	7
90	40	7	0.0	0.0	0	0
100	36	4	4.0	2.0	6	7
120	30	8	0.0	0.0	0	0
150	24	1	4.0	2.0	6	5
180	20	5	0.0	0.0	0	5